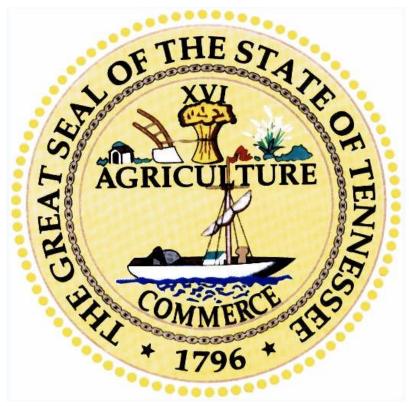
## TECHNICAL REPORT

# INTERSTATE 40 LANE ADDITIONS FROM CENTRAL PIKE TO EAST OF SR-109 WILSON COUNTY PIN 114169.00



PREPARED BY

ARCADIS

FOR THE

TENNESSEE DEPARTMENT OF TRANSPORTATION

PROJECT PLANNING DIVISION

FINAL REPORT
APRIL 12, 2011

#### **Table of Contents**

Introduction	1
Existing Transportation Conditions	1
Proposed Improvements	2
Underpass Clearances	3
Ramp/Weave Analysis	4
Design Exceptions	4
Summary	4
Project Photos	5

#### **List of Figures**

- 1. Area Map
- 2. Location Map
- 3. 2033 Ramp/Weave Results

#### **Appendices**

- A Projected Traffic Volumes
- B Automatic Traffic Recorder Information
- C Functional Plans
- D Bridge Profiles
- E Capacity Analysis
- F Design Exception

Project Description: Interstate 40 Lane Additions From Central Pike to East of SR-109

Wilson County, Tennessee

#### Introduction

The purpose of this study is to provide a technical evaluation of proposed modifications to Interstate 40 (I-40) in Wilson County. Approximate project limits are the Central Pike underpass (LM 2.02) to approximately one (1) mile east of the SR-109 eastbound onramp (LM 10.52). Total length of project study area is approximately 8.5 miles. An area map is contained on Figure 1 and a location map depicting the project study area is provided on Figure 2.

This project is part of the Nashville Area MPO Fiscal Years 2011-2015 Transportation Improvement Program (TIP). The TIP project number is 2011-72-107.

#### **Existing Transportation Conditions**

Within the project area, I-40 is a multi-lane highway facility with twelve (12) foot lanes and ten (10) to twelve (12) foot shoulders, and a 2011 Annual Average Daily Traffic (AADT) volume of 70,950 vehicles. Between Central Pike and SR-171, I-40 consists of four (4) lanes, including one (1) high occupancy vehicle (HOV) lane in each direction, with a median barrier. Through the SR-171 interchange and extending approximately 1.7 miles east, I-40 consists of three (3) lanes, including an HOV lane in each direction with a median barrier. The remaining length of the project consists of two (2) lanes in each direction with a depressed median. Near LM 7.43, I-40 crosses Wilson Creek on parallel bridges spans.

Three (3) interchanges are located within the project. Exit 226 to SR-171, Mt Juliet Road (LM 3.12) is basically a diamond interchange with a cloverleaf added for the eastbound I-40 to northbound SR-171 movement. I-40 eastbound under the SR-171 overpass has a minimum travel lane vertical clearance of 16.21 feet, minimum 8.6-foot paved inside shoulder width and eighteen (18) foot wide paved outside shoulders. Westbound I-40 has a minimum travel lane vertical clearance of 18.68 feet, nine (9) foot minimum paved inside shoulder width and seventeen (17) foot wide paved outside shoulders.

Exit 229 to Beckwith Road (LM 6.28) is a partial cloverleaf configuration. I-40 eastbound under the Beckwith Road overpass has a minimum travel lane vertical clearance of 18.21 feet. Westbound I-40 has a minimum travel lane vertical clearance of 17.49 feet. Inside paved shoulder widths are a minimum 8.6 feet and outside paved shoulder widths are a minimum 10.7 feet in both directions.

Exit 232 to SR-109, Gallatin (LM 9.10) is basically a diamond interchange with a cloverleaf added for the eastbound I-40 to northbound SR-109 movement. I-40 eastbound under the SR-109 overpass has a minimum travel lane vertical clearance of 18.12 feet, with a minimum 13.1-foot paved inside shoulder and a minimum 13.5-foot paved outside shoulder. Westbound I-40 has a minimum travel lane vertical clearance of 15.94 feet, with a minimum 3.7-foot paved inside shoulder and 10.6-foot paved outside shoulder. See project photos for areas near the three (3) existing underpasses.

Truck parking areas are located on both the eastbound and westbound I-40 lanes between SR-171 and Beckwith Road. The truck parking area for eastbound traffic is located approximately 1,300 feet east of the SR-171 eastbound entrance ramp, with a connecting auxiliary lane forming a weave. No auxiliary lane is existing on the westbound side, thus there is no weave.

Project Description: Interstate 40 Lane Additions From Central Pike to East of SR-109 Wilson County, Tennessee

The Project Planning Division of TDOT developed 2033 AADT Volumes and Design Hour Volumes for the project. From these projections it is expected that the traffic on this section of I-40 will increase by 35 percent, to an AADT of 95,740 by 2033. This data is contained in Appendix A.

#### **Proposed Improvements**

The proposed I-40 improvements will provide four (4) twelve (12) foot wide travel lanes (including one [1] HOV lane) in each direction, with a median barrier from Central Pike to SR-109. Generally, ten (10) foot paved outside shoulders and twelve (12) foot wide paved inside shoulders will be provided. Desirable lengths according to AASHTO A Policy on Geometric Design of Highways and Streets, 2004 Edition shall be used for acceleration and deceleration lanes and tapers. All work must be completed within existing right-of-way limits utilizing any measures necessary.

From Central Pike to 1.5 miles east of the SR-171 overpass, the I-40 improvements will consist of milling and overlaying the existing three lanes and adding a twelve (12) foot wide travel lane and ten (10) foot wide paved outside shoulder in each direction. The eastbound auxiliary lanes and a concrete barrier wall to SR-171 will require reconstruction. No improvements are anticipated to the inside shoulders or existing median barrier.

The remaining 5.5 miles consist of adding a twelve (12) foot HOV lane, twelve (12) foot wide inside shoulder and median barrier between the east and west bound lanes. Additionally, ten (10) to twelve (12) feet of roadway widening and ten (10) feet of shoulder paving are required adjacent to the existing outside travel lanes. All remaining pavement will be milled and overlaid within this section.

Near LM 7.43, I-40 crosses Wilson Creek on parallel bridge spans. The proposed improvements will require widening the 150-foot-long spans approximately fifty (50) feet to the inside and five (5) to ten (10) feet along the outside. A combined total of seven (7) box bridges and culverts are located within the project area. It is anticipated that at some locations box/culvert extension and/or barrier protection will be required, and should be evaluated during design.

These improvements will result in four lanes each direction separated by a median barrier per TDOT standard drawing RD01-TS-5B. Typical roadway sections are provided in the functional plans. New pavement markings will be installed for the entire length of the project. No significant interchange reconfigurations are anticipated during this project, however 800 linear feet of Leeville Road will require relocation near LM 8.5.

TDOT Project Planning Division has an existing Automatic Traffic Recorder (ATR #34) located at the end of the Beckwith Road Interchange westbound on-ramp (near LM 5.91). This ATR presently collects traffic data for both eastbound and westbound directions. The planned I-40 improvements will require an additional ATR for collecting the eastbound traffic. The existing ATR (located westbound) will require replacement due to the proposed widening. Additional information has been provided in Appendix B.

TDOT Maintenance is proposing a new salt storage building in the eastbound truck parking area. The salt storage facility is contingent on funding and should not impact the design or construction of this project. A

Project Description: Interstate 40 Lane Additions From Central Pike to East of SR-109 Wilson County, Tennessee

combined HOV enforcement and emergency vehicle/police refuge area is also under consideration for inclusion in this project.

Guide signage along this section of I-40 has been addressed. Several signs will require relocation based on the proposed widening and have been noted. All signs identified during design that do not meet the 2009 Manual on Uniform Traffic Control Devices (MUTCD) retro-reflectivity requirements shall be replaced. The functional plans note any proposed and/or modification to existing guide signs.

Functional plans for the proposed improvements are contained in Appendix C.

#### **Underpass Clearances**

Three (3) I-40 underpasses at SR-171 (LM 3.12), Beckwith Road (LM 6.28) and SR-109 (LM 9.10) were field surveyed and bridge profiles created. Each bridge was profiled along the east and west side of the structure. A full profile across the travel lanes and along the lowest member of the structure was generated. Horizontal and vertical control was established at each crossing using Global Positioning System (GPS) methods. The datum collected was Tennessee State Grid coordinates NAD 83 for horizontal and NAVD 88 for vertical. Control points were established using the OPUS Rapid Static survey process. Data points were collected using traditional survey methods in combination with reflectorless technology to locate the bottom of the bridge structure. The control points are noted on the functional plans and bridge profiles showing both present and proposed roadway configurations are provided in Appendix D.

TDOT standard drawing RD01-TS-5B requires a minimum twelve (12) feet from inside edge of travel lane to centerline median barrier (using a ten [10] foot minimum inside shoulder width) and minimum ten (10) foot wide paved outside shoulders. Minimum acceptable vertical bridge clearance per the AASHTO A Policy on Geometric Design of Highways and Streets, 2004 Edition is sixteen (16) feet, including usable shoulder width. A summary of the minimum shoulder widths and minimum vertical clearances for the proposed improvements at each underpass is shown below:

	SR-171 Underpass (LM 3.12) (feet)	Beckwith Road Underpass (LM 6.28)	SR-109 Underpass (LM 9.10)
Minimum Vertical Clearance			
East Bound Travel Lane	15.65	17.38	17.76
East Bound Shoulder	15.05	17.45	17.63
West Bound Travel Lane	18.41	17.22	15.74
West Bound Shoulder	17.06	17.36	15.52
Shoulder Width			
East Bound Inside	4.0	10.7	10.7
East Bound Outside	10.3	12.0	12.0
West Bound Inside	10.0	10.7	11.3
West Bound Outside	11.3	12.0	12.0
Red text indicates minimum standard no	ot met.		

Project Description: Interstate 40 Lane Additions From Central Pike to East of SR-109

Wilson County, Tennessee

#### Ramp/Weave Analysis

Highway Capacity Software (HCS) analyses were made for the proposed improvements at the eastbound diverge to SR-171, the westbound merge from SR-171, and the eastbound weave between the SR-171 on ramp and the truck parking area. These analyses were made assuming fifteen (15) percent of the through volume is occupying the HOV lane. The fifteen (15) percent is derived from data collected by TDOT for I-65 in Williamson County. Printouts of the capacity analysis are provided in Appendix D. Capacity results are shown on Figure 3.

Capacity analysis indicated that by 2033, the eastbound diverge to SR-171 will reach an LOS F in the P.M. peak and that the westbound merge from SR-171 will reach an LOS F in the A.M. These operational deficiencies are mainly due to the heavy through traffic volume between the Mt. Juliet area and Nashville. These basic freeway sections also have operational deficiencies by 2033, with an eastbound LOS E and a westbound LOS F. Improving this deficiency would require a total of five (5) lanes (including one HOV lane) each direction to accommodate the projected traffic volumes. The weave between SR-171 and the truck parking area will reach an LOS D in the P.M. West of SR-171 all basic freeway sections would be expected to operate at an LOS C or better.

#### **Design Exceptions**

The majority of the project does not require design exceptions, however three locations were identified that have or will have less than minimum horizontal and/or vertical clearance. At the SR-171 (Mt Juliet Road) underpass, the width of the proposed eastbound inside shoulder is four (4) feet. A shoulder width design exception is contained in Appendix D. As shown in the bridge sections, the vertical clearance at this location will be 15.05 feet. Options to achieve a sixteen (16) foot minimum vertical clearance shall be identified and evaluated during design.

At the SR-109 interchange underpass the existing westbound vertical clearance is 15.52 feet. Additionally, the proposed twelve (12) foot wide eastbound outside shoulder may require an approximately nine (9) foot high retaining wall. Options to achieve sixteen (16) foot minimum vertical clearance and a twelve (12) foot wide outside shoulder shall be identified and evaluated during design.

Bridge profiles including all critical dimensions are provided in Appendix D.

#### **Summary**

I-40 project limits are the Central Pike underpass (LM 2.02) to approximately one (1) mile east of the SR-109 eastbound onramp (LM 10.52) or approximately 8.5 miles. The existing Annual Average Daily Traffic (AADT) volume is 70,950 vehicles and by 2033 the traffic should increase by 35 percent to an AADT of 95,740.

Capacity analysis was performed at the Mt Juliet Ramps and the weave located between Mt Juliet on ramp and the eastbound truck parking area. Below is a list of the results:

 Diverge to SR-171 will reach an LOS F in the P.M. peak (improving the LOS will require an additional travel lane between Mt. Juliet and Nashville due to the heavy through traffic volume.) Project Description: Interstate 40 Lane Additions From Central Pike to East of SR-109 Wilson County, Tennessee

- Merge from SR-171 will reach an LOS F in the A.M. peak (improving the LOS will require an additional travel lane between Mt. Juliet and Nashville due to the heavy through traffic volume.)
- Weave between SR-171 and the truck parking area will reach an LOS D in the P.M. West of SR-171

The proposed I-40 improvements will provide eight (8) twelve (12) foot wide travel lanes, including one (1) HOV lane each direction, with ten (10) foot wide paved outside shoulders and twelve (12) foot wide paved inside shoulders from Central Pike to SR-109. The improvements will require one shoulder width design exception at the SR-171 underpass, bridge widening at Wilson Creek near LM 7.43, replacing the ATR#34 near LM 5.95 and relocation of Leeville Road.

## **Project Photographs**

#### **TECHNICAL REPORT**

Interstate 40 Lane Additions From Central Pike to East of SR-109 Wilson County

Photos Taken: 01/27/2011



Photo 1
I-40 at SR-171/Mt Juliet
Road Underpass Looking
West



Photo 2
I-40 at SR-171/Mt Juliet
Road Underpass Looking
West

## **Project Photographs**

#### **TECHNICAL REPORT**

Interstate 40 Lane Additions From Central Pike to East of SR-109 Wilson County

Photos Taken: 01/27/2011



Photo 3

I-40 at Beckwith Road Underpass Looking East



Photo 4

I-40 at Beckwith Road Underpass Looking West

## **Project Photographs**

#### **TECHNICAL REPORT**

Interstate 40 Lane Additions From Central Pike to East of SR-109 Wilson County

Photos Taken: 01/27/2011



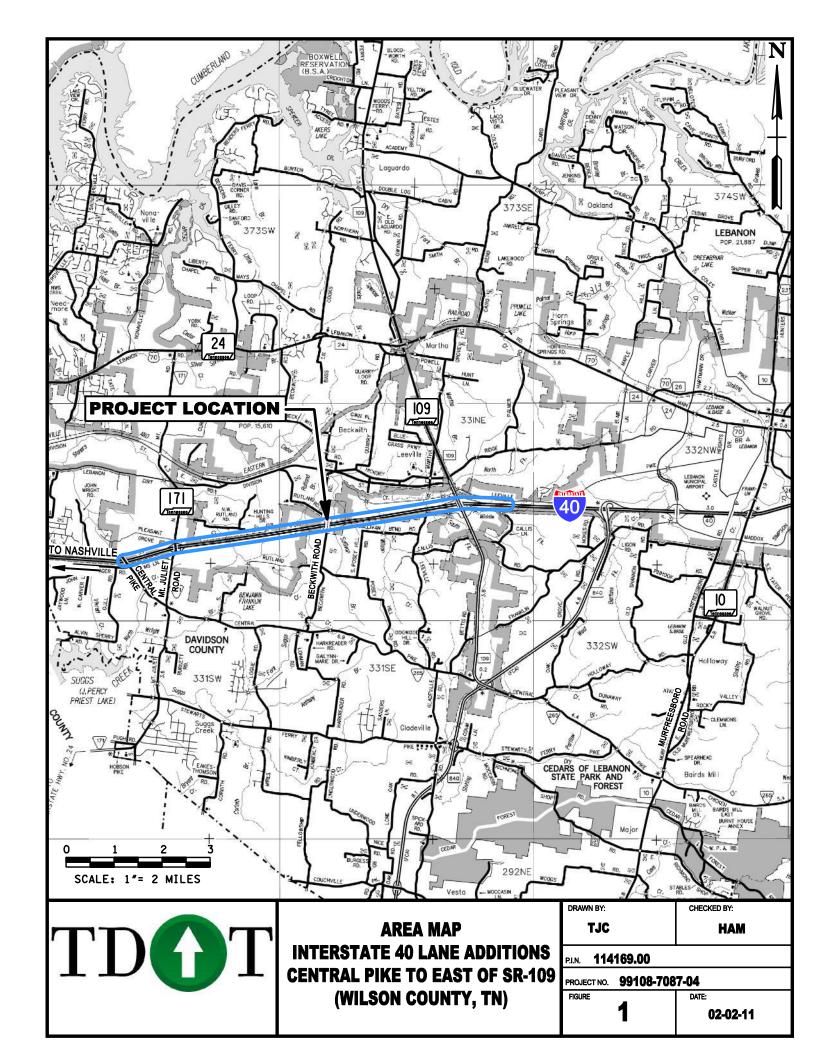
#### Photo 5

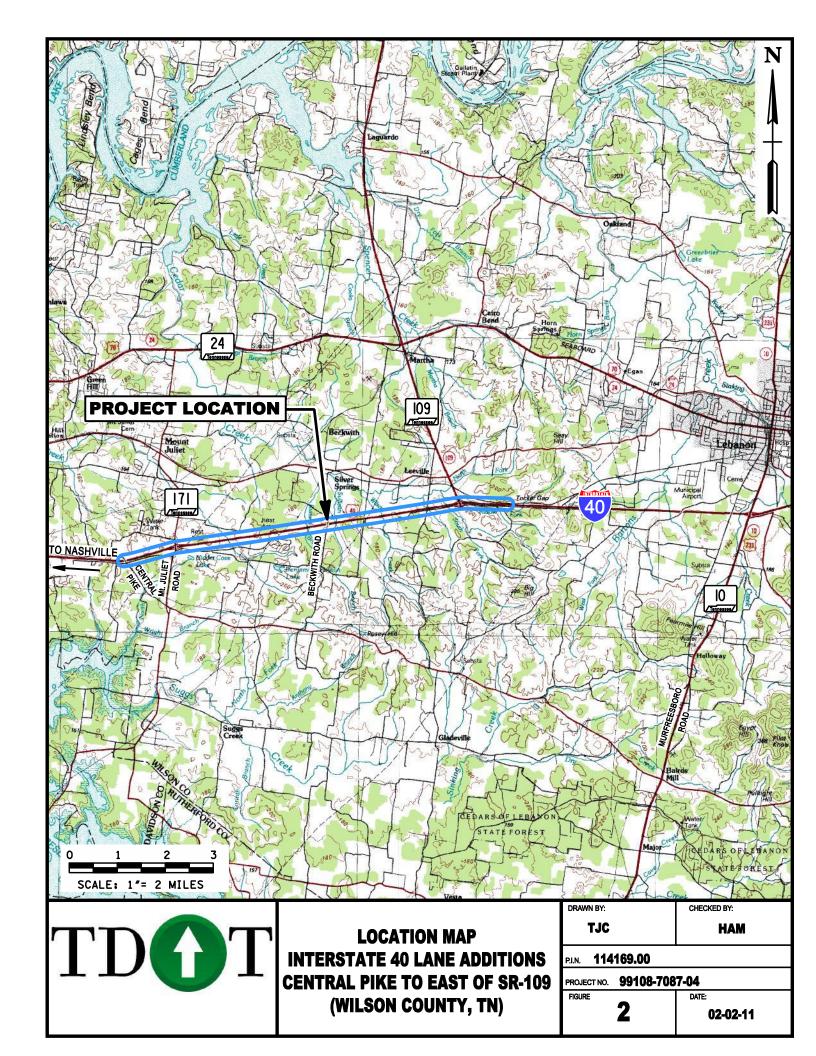
I-40 at SR-109 Underpass Looking East

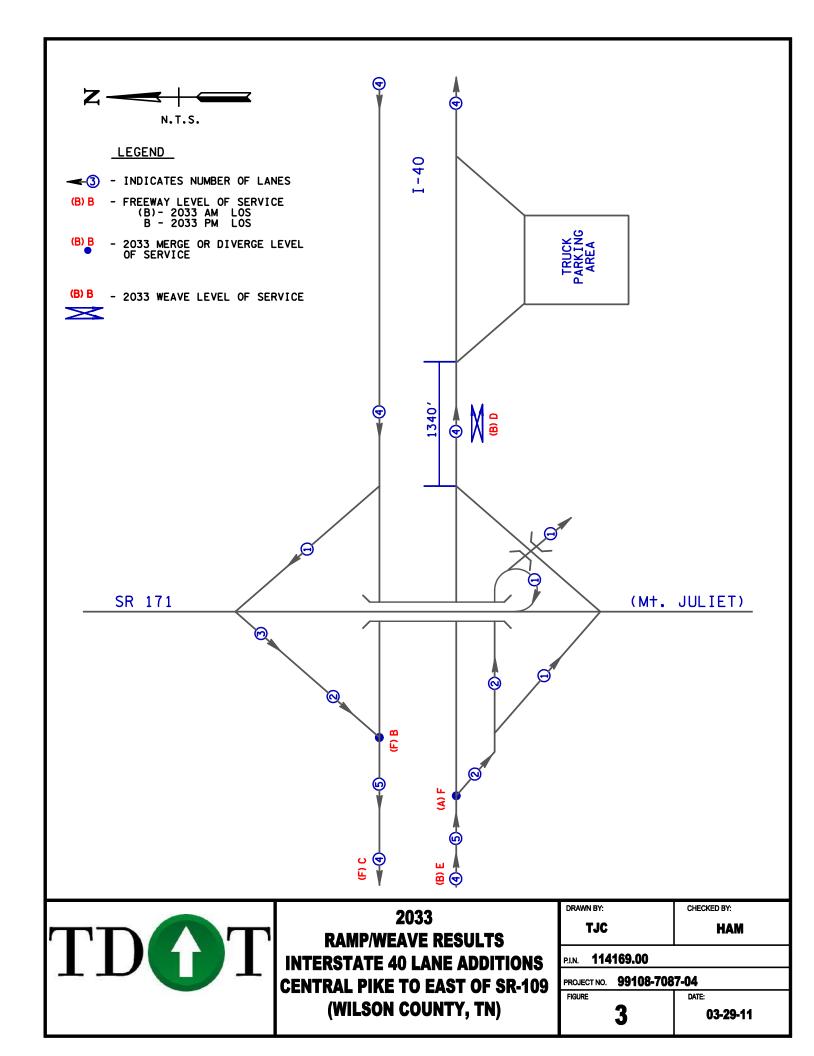


#### Photo 6

I-40 at SR-109 Underpass Looking West







Project Description: Interstate 40 Lane Additions

Technical Report

From Central Pike to East of SR-109 Wilson County, Tennessee

Appendix A

Projected Traffic Volumes

#### TENNESSEE DEPARTMENT OF TRANSPORTATION PROJECT PLANNING DIVISION

<b>PROJECT</b>	NO.:					ROUTE:	1-40			
COUNTY:	V	VILSON				CITY:	MT. JUI	JET		
<b>PROJECT</b>	PIN NUM	IBER: 114	169.00							
<b>PROJECT</b>	DESCRIP	PTION: W	IDENING	FRON	M WEST	OF S.R. 171 T	O EAST	OF S.R. 10	09.	
		[D	ESIGN BU	JILD]				,		
DIVISIO	ON REO	UESTING								
						PAVEMEN	T DESI	GN		
MAINTE	NANCE					STRUCTU				ī
PLANNI				ī		SURVEY &		in		i
		MENT & A	DM.	ī		TRAFFIC S				<b>i</b>
		& AERO.		์ โ		OTHER				$\overline{\times}$
		ROGRAMME	ED FOR CO	ONST	RUCTION			21 21 11 202		
		ING DATE:				2012				_
TRAFFI	C ASSI	GNMENT	•							
							DE	SIGN	DES	SIGN
							1	DWAY	AVE	RAGE
BASE Y	/EAR		DES	IGN Y	YEAR		% TR	RUCKS	DAILY	LOADS
AADT	YEAR	AADT	DHV	%	YEAR	DIR.DIST.	DHV	AADT	FLEX	RIGID
72,540	2013	95,740	8,308	8	2033	60-40	12	18	4,052	6,350
										-
				-	-					
REQUEST	ED BY:	NAME	DANIE	LLE	HAGEWO	OOD		DATE	3/15/11	
		DIVISION			ANAGEN			_		
		ADDRESS			NNIAL B					
					TN 3720					
					-	1	1			
REVIEWE	ED BY:	TONY ARM	ASTRONG		one	Aunath		DATE	3.23	-11
		TRANSPOR	RTATION	MAN	AGER 1	-				
		SUITE 1000	, JAMES I	K. PO	LK BUILI	DING/				
				1	)	1//			,	
APPROVE	ED BY:	BILL HART	L C	15	21 1	faut		DATE	3/23	/11
		TRANSPOR	RTATION	MAN	AGER 2					
		SUITE 1000	), JAMES I	K. PO	LK BUILI	DING				
COMM	RNTS									

THIS TRAFFIC WAS UPDATED FROM THE PREVIOUS PROJECT PREPARED FOR BRIAN HURST IN PLANNING DATED 1-18-2011.

cc: LIA OBAID: CONSTRUCTION OFFICE

DHV'S ARE NOT REQUIRED FOR SIDE ROADS LESS THAN 1000 AADT.

NOTE: FOR BRIDGE REPLACEMENT PROJECTS, ADLS ARE NOT REQUIRED FOR AADT'S OF 1000 OR LESS AND PERCENTAGE OF TRUCKS OF 7% OR LESS.

## TENNESSEE DEPARTMENT OF TRANSPORTATION TRAFFIC PLANNING AND STATISTICS OFFICE

PROJECT	NO.:				ROUTE	NO.: 1-40	)
COUNTY:	WILSON				CITY: N	AT. JULIET	-
PROJECT	DESCRIPTION:	WIDENING FROM	WEST OF S.F	R. 171 To	OEASTO	F S.R. 109	

#### Interstate

#### Pavement Structural Design

Calculation of Equivalent Daily 18 Kip Single Axle Loads

		ADT	Flexible		Rigid	
Тур	oe Vehicle	(No. Counted)	18-kip Factor	ADL	18-kip Factor	ADL
Pass. c	ars and					
motorcy	/cles (1-2)	55,356	0.001	55	0.001	55
Pick-up	, Panel,					
Van	(3)	13,639	0.004	55	0.005	68
	Buses (4)	327	0.300	98	0.300	98
Sing.	2-axle,					
	6-tire (5)	2,656	0.170	452	0.170	452
Unit	3-axle or					
	more (6-7)	793	0.700	555	1.000	793
	4-axle (8)	535	0.700	375	0.780	417
Comb.	5-axle or					
	more (9-13)	10,834	1.100	11,917	1.780	19,285
	Totals					
(20	23 AADT)	84,140		13,507		21,168

Suggested Percentages of Trucks in Design Lane

	4 Lane	6 Lane	8 Lane
5,000 or less ADT	90%	75%	70%
5,000 - 10,000 ADT	80%	70%	65%
10,000 - 15,000 ADT	75%	65%	60%
15,000 - 20,000 ADT	75%	65%	55%
20,000 - 30,000 ADT	70%	60%	50%
30,000 Plus ADT	65%	60%	50%

No. of Lanes: 6
% Trucks in Design Lane: 60%

ADL in Design Lane:

FLEX: 0.5 X 0.60 X 13506.5 = 4,052 RIGID: 0.5 X 0.60 X 21168.0 = 6,350

ADL Calculations By: TONY ARMSTRONG

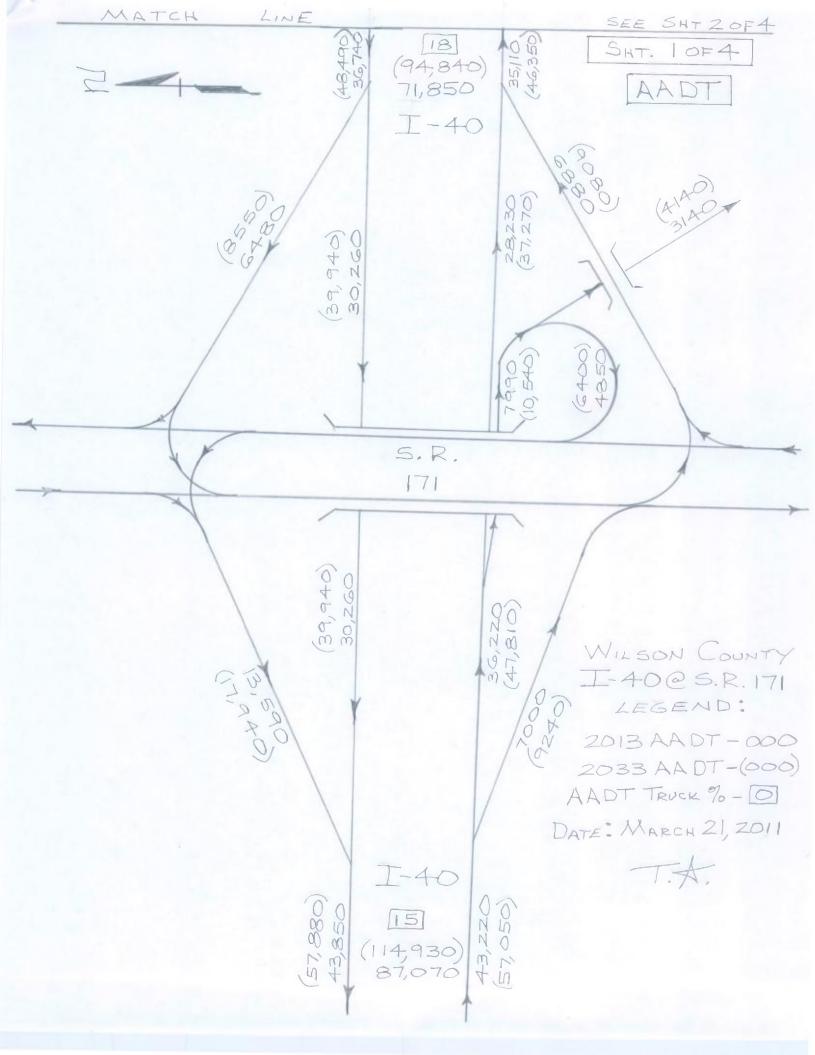
Reviewed By:

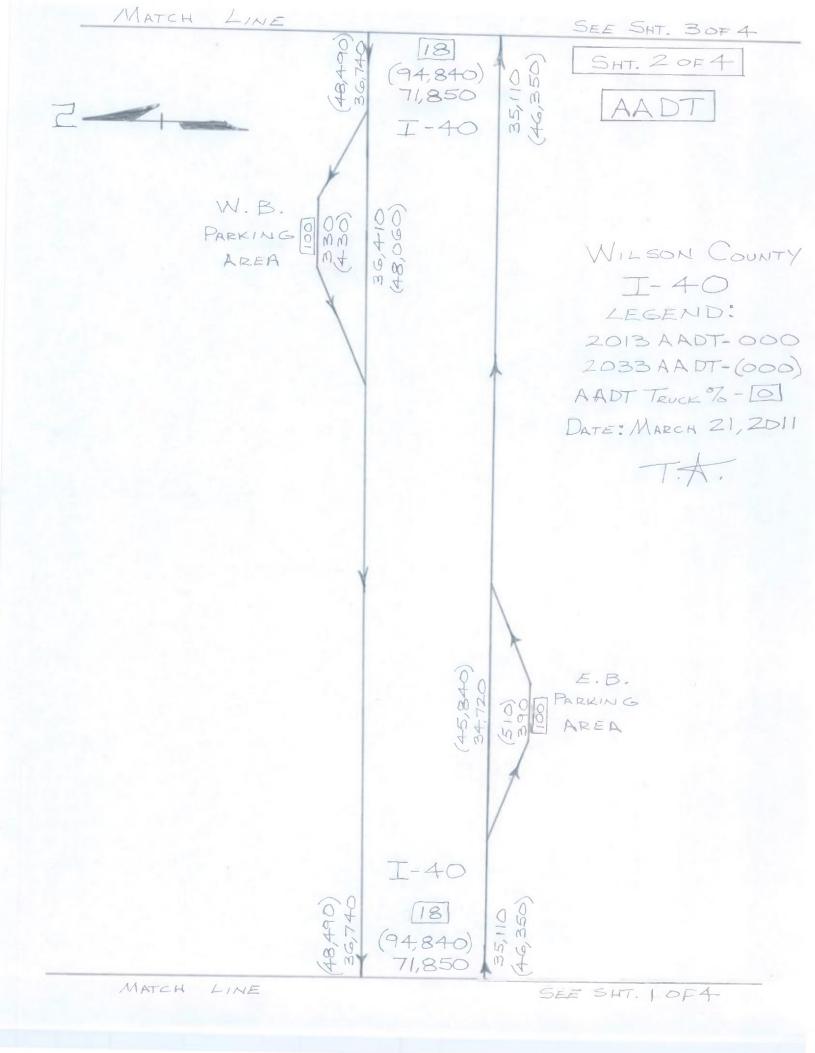
[REV. 11-6-06]

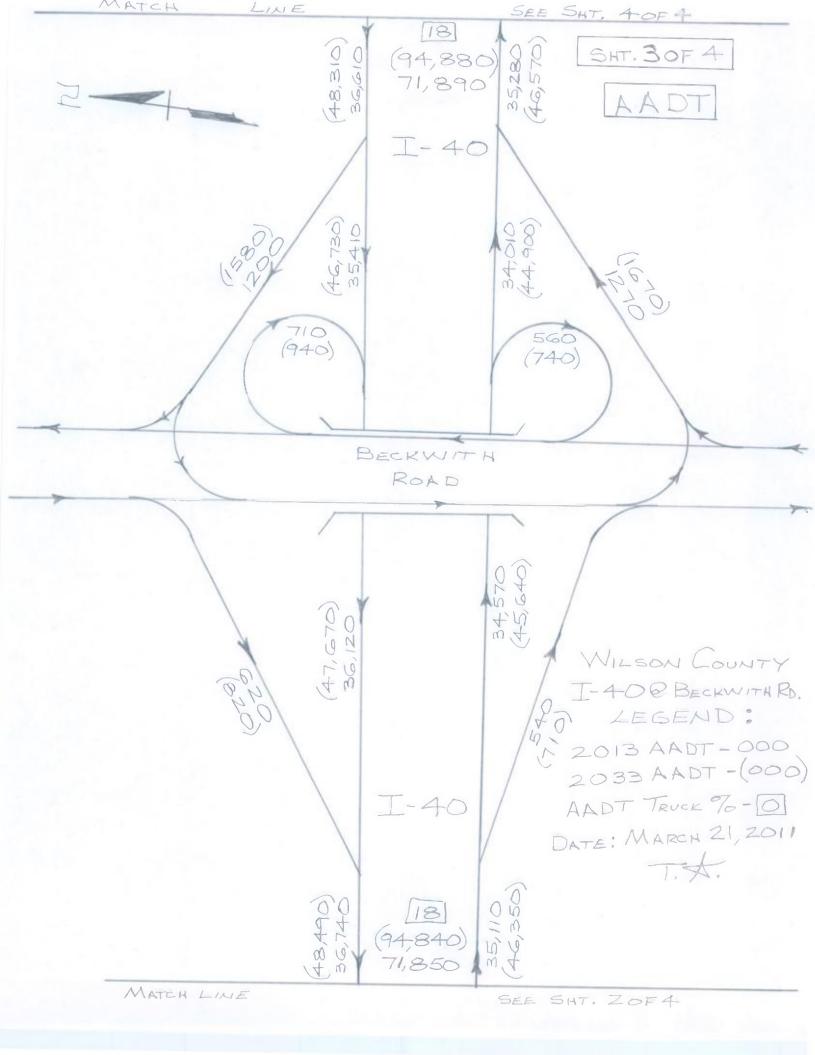
Date: 3/23/2011

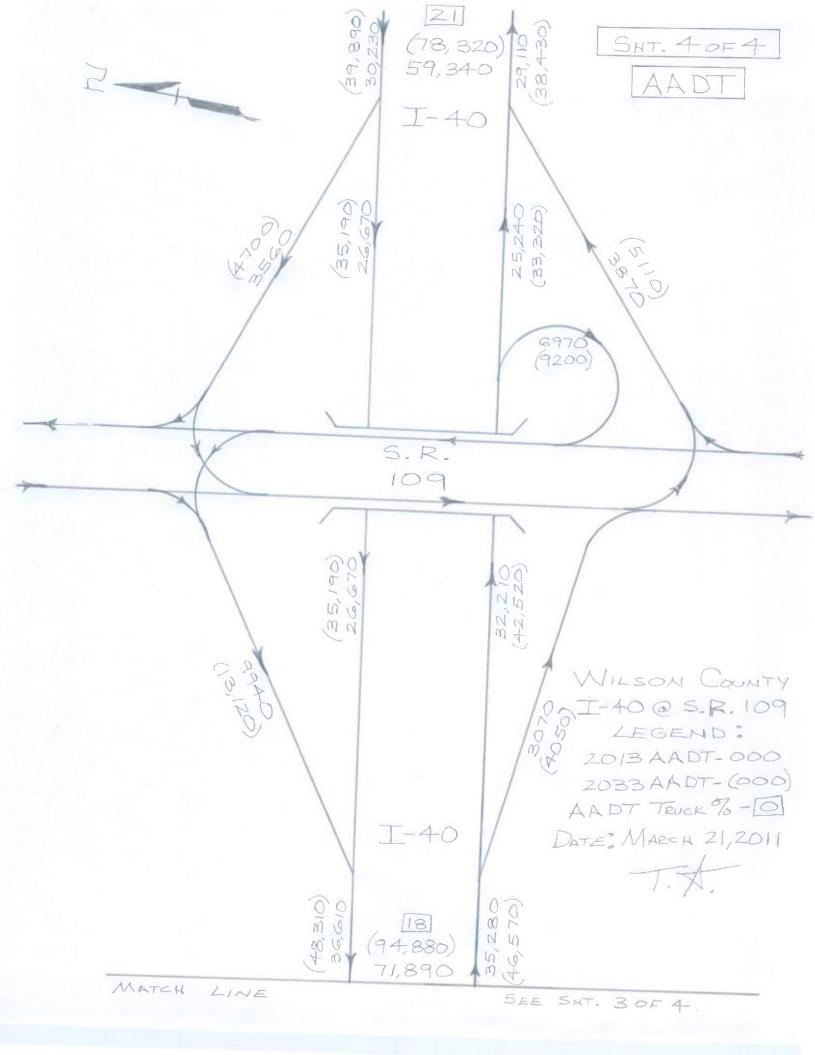
Date: 3/23/2011

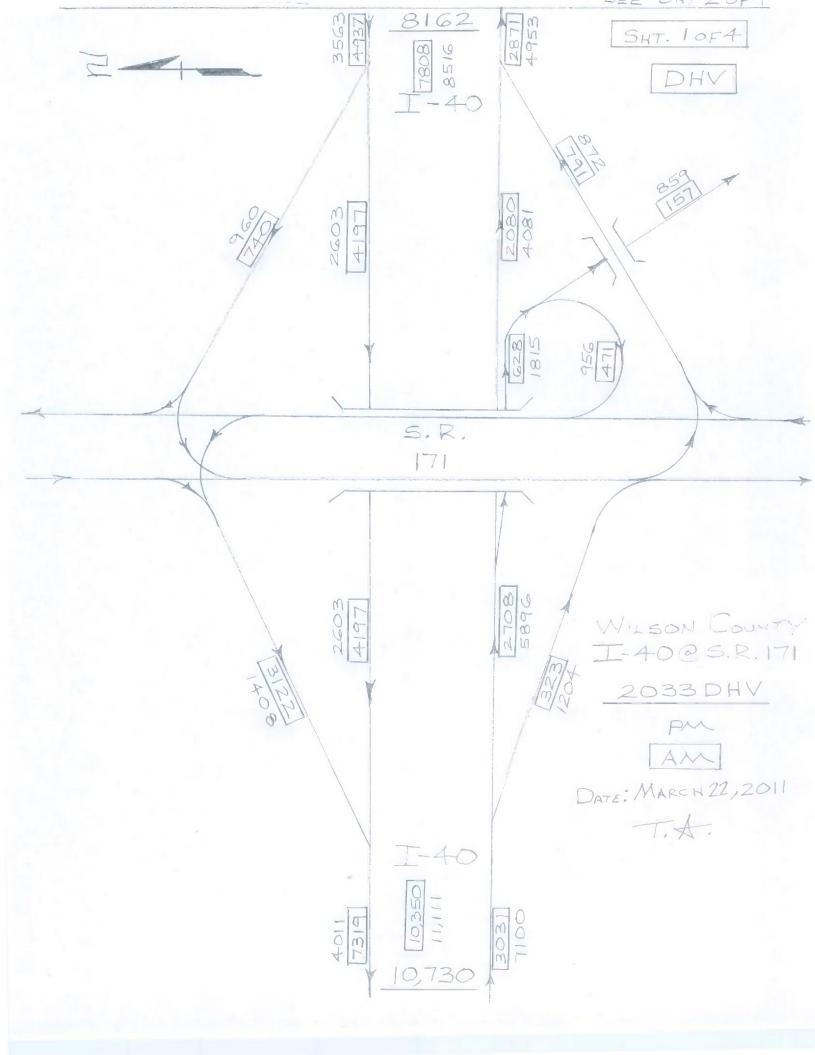


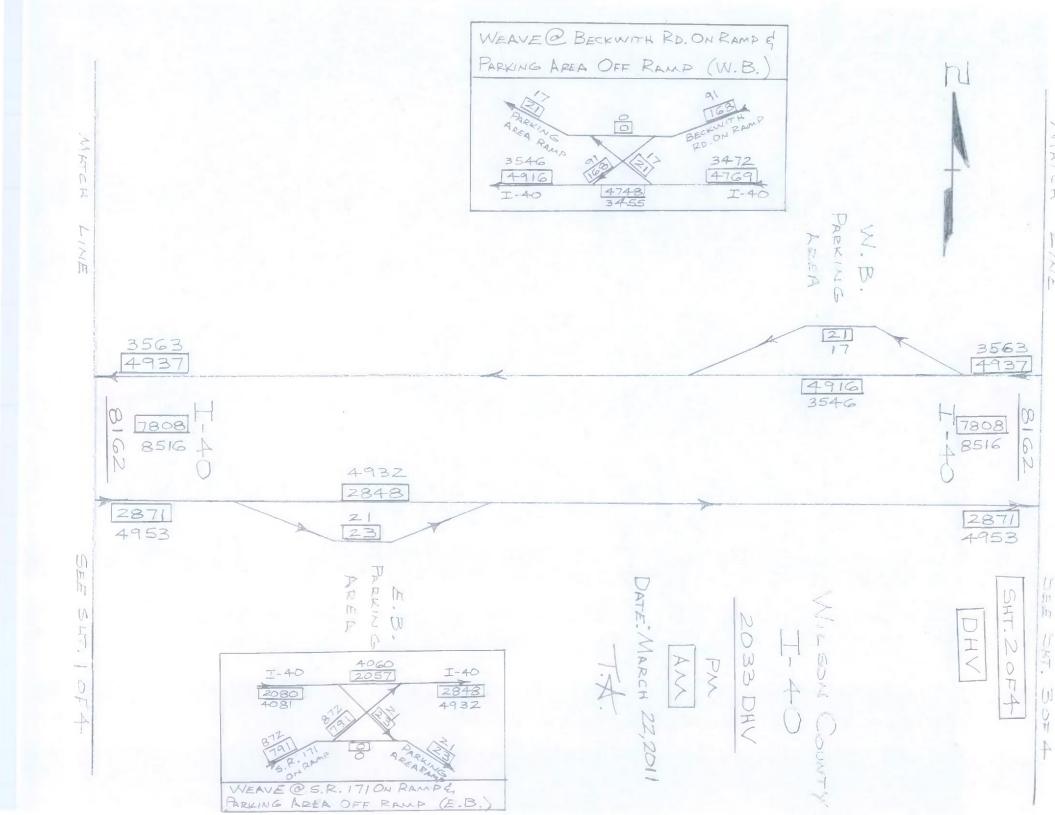


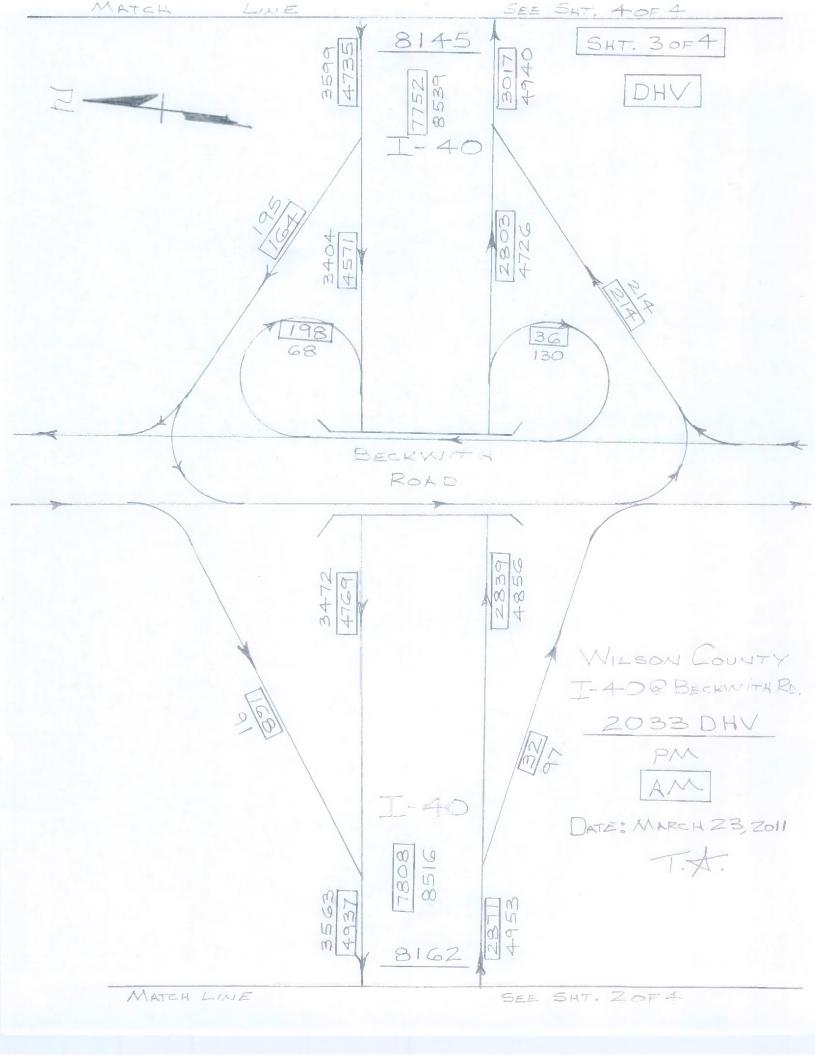


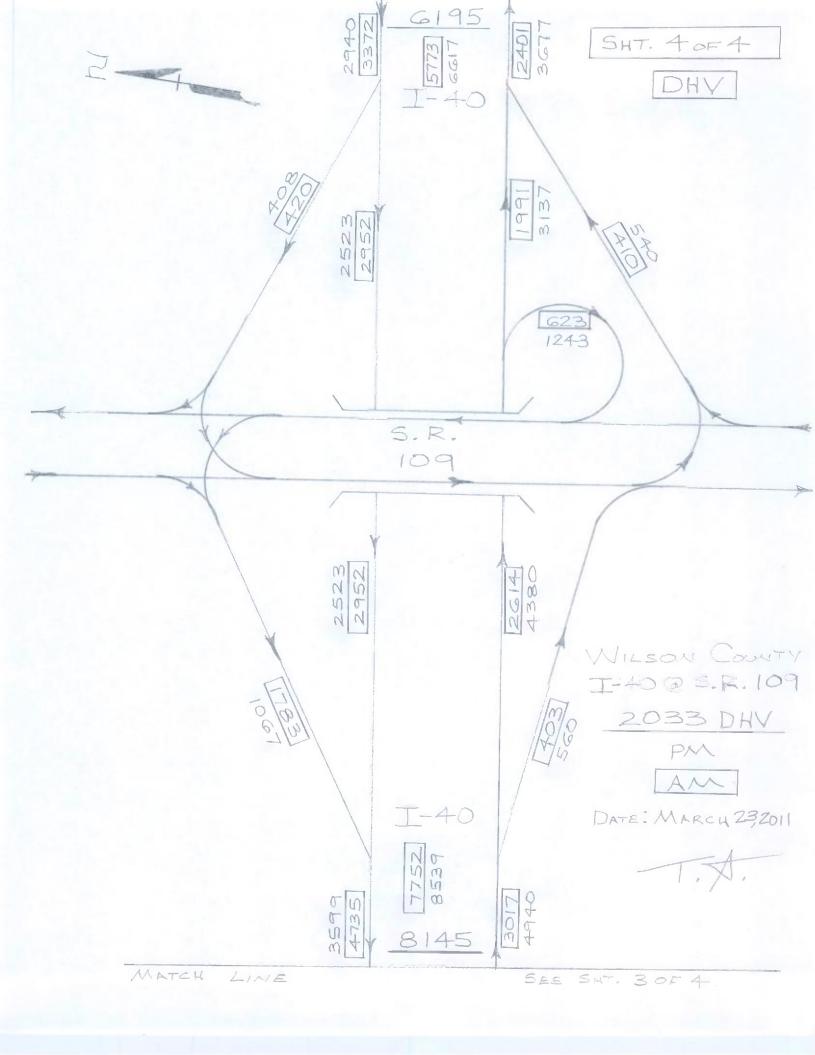












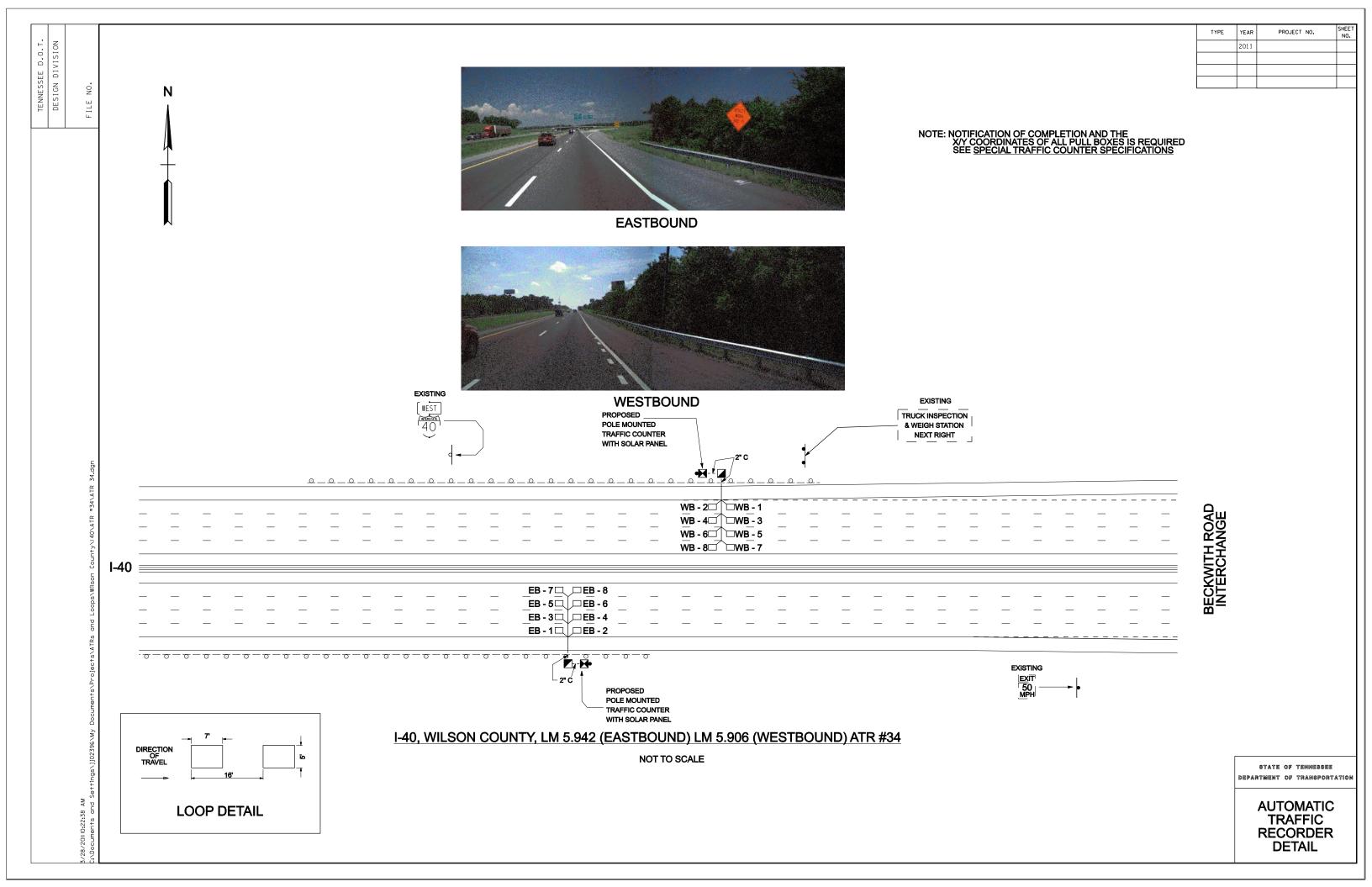
Project Description: Interstate 40 Lane Additions

Technical Report

From Central Pike to East of SR-109 Wilson County, Tennessee

Appendix B

Automatic Traffic Recorder Information



#### SPECIAL TRAFFIC COUNTER SPECIFICATIONS

CONTRACTOR SHALL SUPPLY AND INSTALL ALL RELATED EQUIPMENT SO THAT ONCE A TRAFFIC COUNTER AND MODEM ARE INSTALLED BY THE STATE'S PLANNING DIVISION'S TRAFFIC COUNTER PERSONNEL, THE COUNT STATION WILL BE FULLY OPERATIONAL. THE CONTRACTOR SHALL MAKE SURE EACH DETECTION LOOP IS OPERATIONAL AT THE COMPLETION OF THE PROJECT.

COUNTER CABINET SHALL NOT TO BE PRE-WIRED FOR A TRAFFIC SIGNAL AND DOES NOT NEED TO INCLUDE A VENTILATION FAN. THE CABINET SHALL INCLUDE ONE (1) SHELVE. THE CABINET SHALL BE POLE-MOUNTED TO A FOUR INCH (4") GALVANIZED, RIGID CONDUIT POLE WITH THE BOTTOM OF THE CABINET AT LEAST 36" ABOVE GROUND LEVEL.

THE CABINET DOOR SHALL BE SEALED WITH A CLOSED CELL NEOPREME GASKET BONDED TO THE INSIDE OF THE DOOR WITH AN OIL RESISTENT ADHESIVE THAT WILL BE POSITIONED TO PREVENT ANYTHING FROM PENTRATING THROUGH THE CRACK BETWEEN THE DOOR AND THE CABINET WHEN THE DOOR IS CLOSED. THE CABINET SHALL BE MOUNTED SO THAT THE CABINET DOOR WILL OPEN TOWARD THE ROADWAY.

INSTALLATION SHALL INCLUDE EXTERNAL GROUNDING OF THE CABINET FOR LIGHTNING PROTECTION. THIS GROUNDING SHALL INCLUDE AN EIGHT FOOT (8') GROUNDING ROD INSTALLED OUTSIDE IN THE GROUND CONNECTED TO A #6 COPPER WIRE TERMINATING TO A GROUNDING BAR INSIDE THE CABINET.

THE 12-VOLT SOLAR PANEL SHALL BE 80-WATT WITH CABINET VOLTAGE REGULATOR. THE SOLAR PANEL SHALL BE MOUNTED EIGHT FEET (8') ABOVE THE CABINET AND ON THE SAME CONDUIT POLE. IT SHALL HAVE A ONE (1") INCH SCHEDULE 40 CONDUIT ATTACHED TO THE POLE COMPLETE WITH A WEATHER HEAD. THIS CONDUIT SHOULD START AT THE TOP OF THE POLE AND EXTEND DOWN THE POLE, INTO THE CABINET.

ALL LOOPS SHALL BE FIVE FEET BY SEVEN FEET (5' X 7') AND TYPICAL IN SIZE WITH THREE (3) TURNS OF LOOP WIRE.

ALL LOOP WIRES AND SOLAR PANEL WIRES SHALL TERMINATE IN THE CABINET CONNECTED TO A TERMINAL STRIP.

EACH LOOP DETECTOR LEAD-IN SHALL BE MARKED WITH LANE IDENTIFICATION, DENOTING THE LANE AND POSITION IN THE LANE FOR EACH LOOP (SUCH AS EB-1 FOR EASTBOUND 1, ECT.).

PAYMENT OF TELEPHONE BILLS SHOULD BE INVOICED TO: DIRECTOR OF PROJECT PLANNING DIVISION SUITE 1000, JAMES K. POLK BUILDING 505 DEADRICK STREET NASHVILLE, TN 37243-0344

FOR QUESTIONS CONCERNING ANY OF THE INSTALLATION OF THE COUNTER EQUIPMENT, PLEASE CONTACT STANLEY DUNN AT 615-350-4571 OR AT <u>STANLEY.DUNN@TN.GOV</u>. CONTRACTOR SHALL NOTIFY STANLEY DUNN TO REPORT THAT WORK HAS BEEN COMPLETED SO HE CAN FINISH THE INSTALLATION.

THE CONTRACTOR SHALL SUPPLY STANLEY DUNN WITH THE X / Y COORDINATES (BASED ON WGS COORDINATES) OF EACH PULL BOX.

#### SPECIAL GENERAL NOTES REQUIRED:

EQUIPMENT AND INSTALLATION OF TRAFFIC SIGNAL ITEMS SHALL COMPLY WITH TDOT STANDARD SPECIFICATIONS, SECTION 730.

DETECTION LOOPS SHALL BE INSTALLED BEFORE THE FINAL SURFACE IS APPLIED.

#### **STANDARD DRAWINGS REQUIRED:**

- RD-L-3 STANDARD LEGEND FOR SIGNALIZATION AND LIGHTING
- T-SG-2 LOOP LEAD-INS, CONDUIT AND PULL BOXES
- T-SG-3 STANDARD NOTES AND DETAILS OF INDUCTIVE LOOPS

#### **ITEMS AND QUANTITIES:**

730-03.21	INSTALL PULL BOX (TYPE B)	EACH	2
730-12.02	CONDUIT 2" DIAMETER (PVC)	L.F.	60
730-14.02	SAW SLOT	L.F.	710
730-14.03	LOOP WIRE	L.F.	3240
730-15.07	CABINET	EACH	2

FOOTNOTE ITEM 730-15.07---MINIMUM SIZE OF CABINET SHALL BE 41" TALL, 25" WIDE AND 18" DEEP. ITEM INCLUDES COMPLETE INSTALLATION PER SPECIAL TRAFFIC COUNTER SPECIFICATION NOTES CONCERNING THE COUNTER CABINET. ITEM INCLUDES THE 4" GALVANIZED, RIGID CONDUIT POLE THAT THE CABINET IS TO BE MOUNTED ONTO. ITEM ALSO INCLUDES COMPLETE INSTALLATION OF THE SOLAR PANEL.

Technical Report

Project Description: Interstate 40 Lane Additions

From Central Pike to East of SR-109 Wilson County, Tennessee

Appendix C

Functional Plans

#### Index Of Sheets

1 ..... TITLE 2 ..... TYPICAL SECTIONS 3-32 ..... LAYOUT SHEETS

## STATE OF TENNESSEE DEPARTMENT OF TRANSPORTATION BUREAU OF ENGINEERING

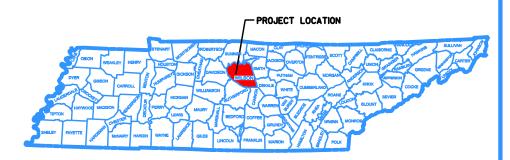
TENN	YEAR	SHEET NO.
TENN.	2011	1
FED. AID PROJ. NO.		
STATE PROJ. NO.	99108-7087-04	

## WILSON COUNTY

INTERSTATE 40 LANE ADDITIONS FROM CENTRAL PIKE TO EAST OF SR-109

STATE HIGHWAY NO.

F.A.H.S. NO.



## 331NW\_ PROJECT LOCATION 840 DAVIDSON 332SW E COUNTY SUGGS 331SW (J.PERCY PRIEST LAKE) CEDARS OF LEBANON STATE PARK AND

SCALE: 1"= 2 MILES

TRAFFIC	DATA
ADT (2011)	70,950
ADT (2031)	93,650
DHV (2011)	8,051
D	60% / 40%
T (ADT)	18%
T (DHV)	12%

U.S. DEPARTMENT OF TRANSPORTA FEDERAL HIGHWAY ADMINISTRAT	
APPROVED₂	
DIVISION ADMINISTRATOR	DATE

DATE:

SEALED BY

CHIEF ENGINEER

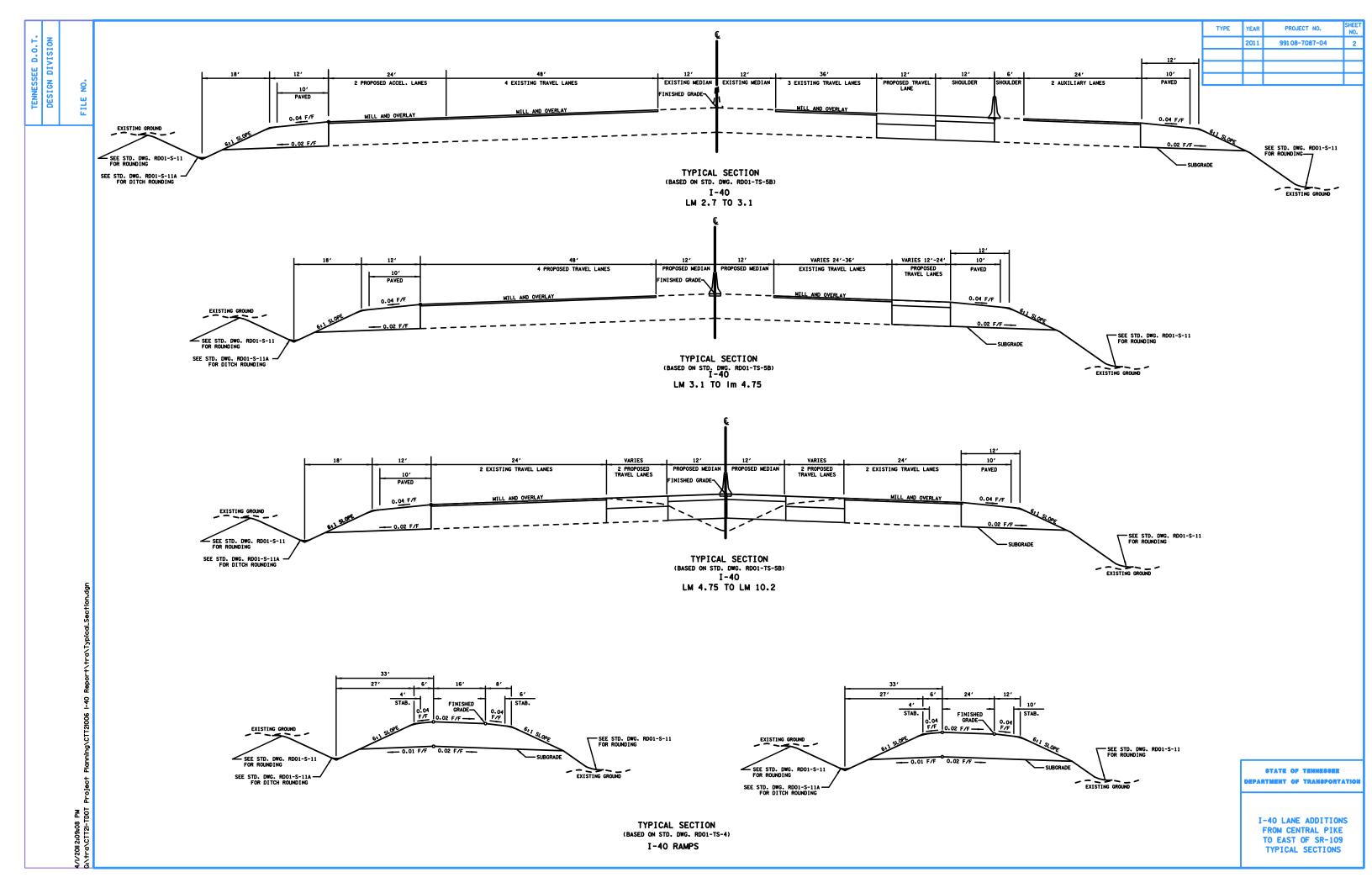
COMMISSIONER

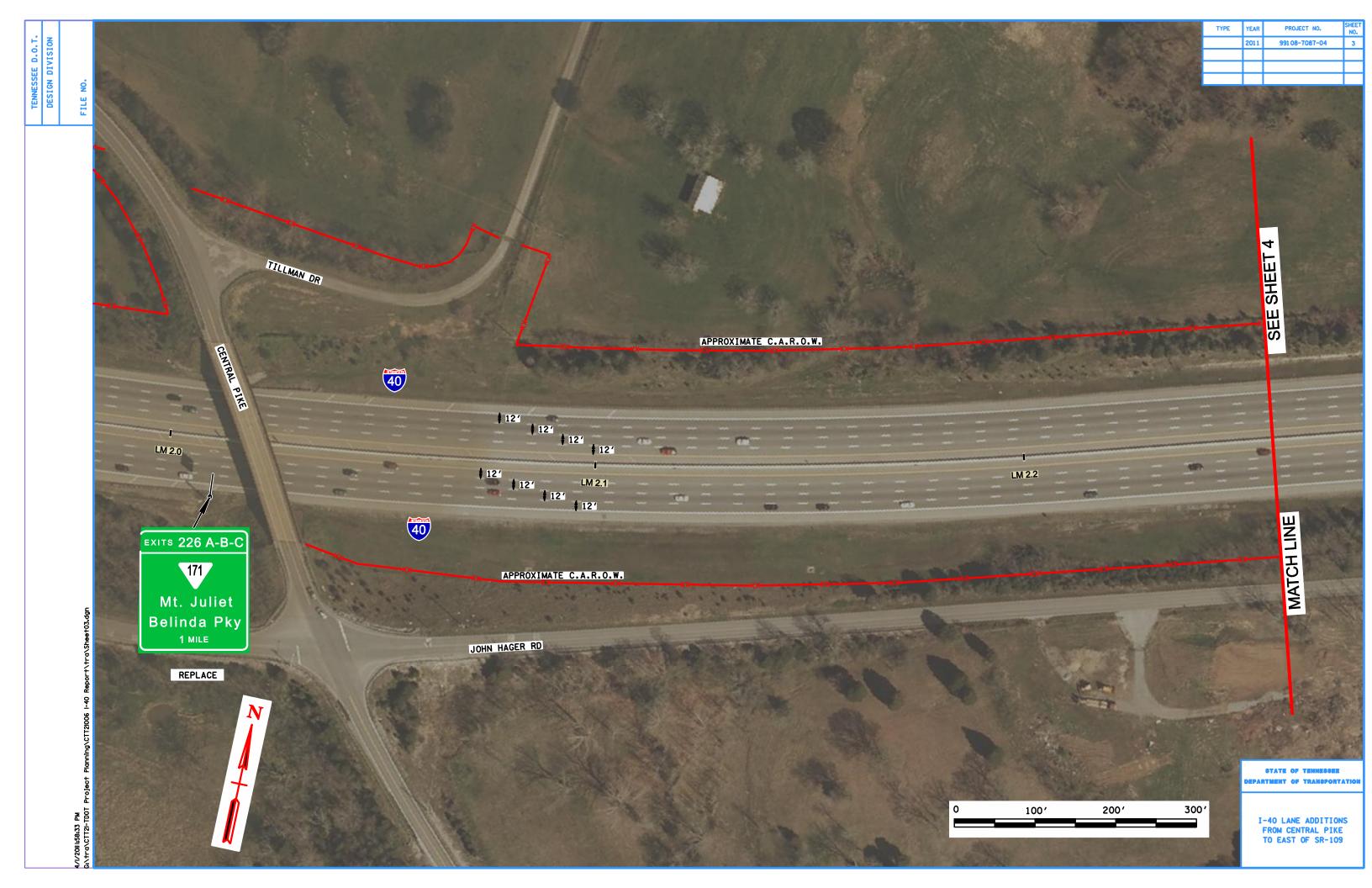
### SPECIAL NOTES

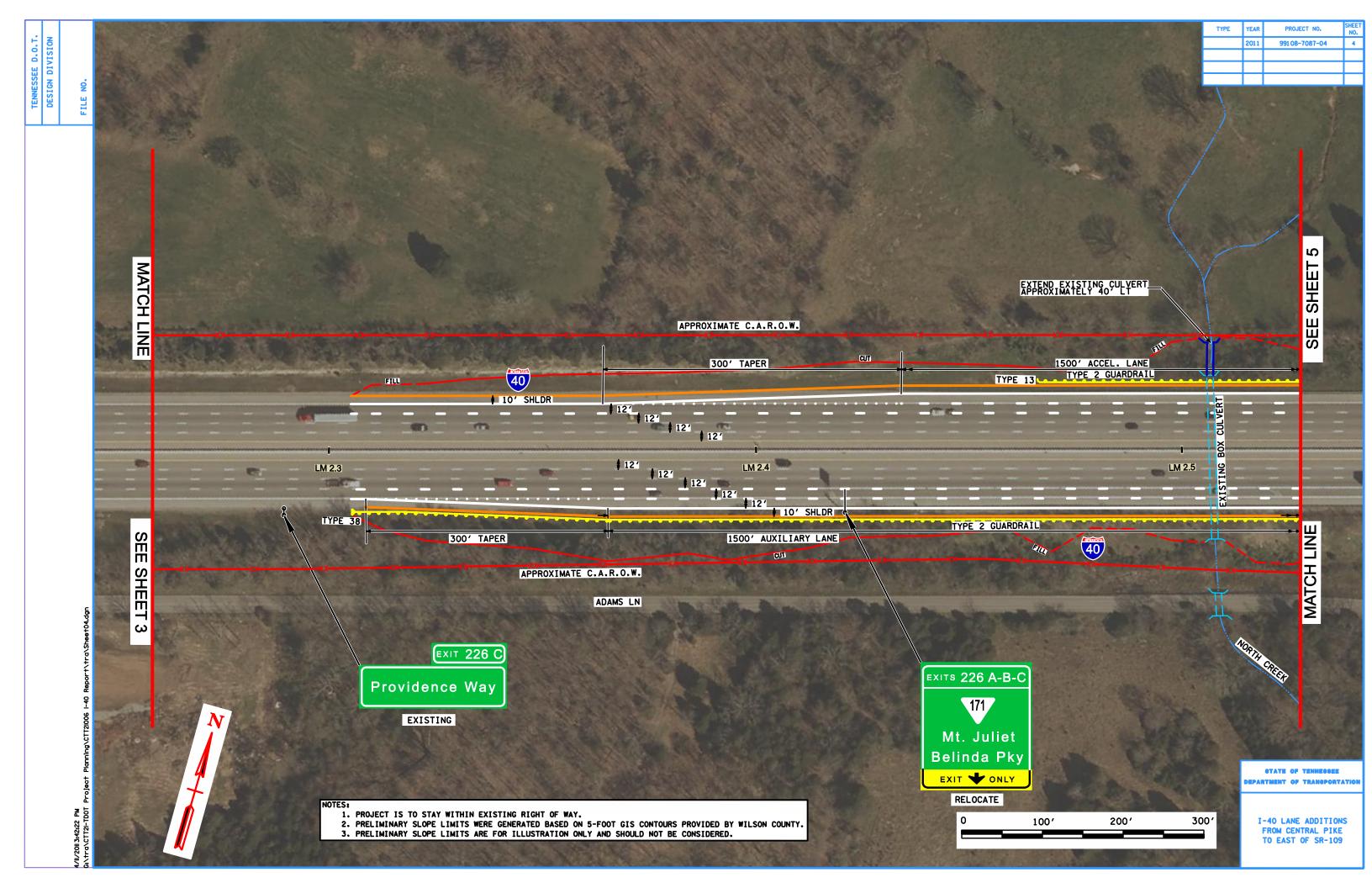
PROPOSALS MAY BE REJECTED BY THE COMMISSIONER IF ANY OF THE UNIT PRICES CONTAINED THEREIN ARE OBVIOUSLY UNBALANCED, EITHER EXCESSIVE OR BELOW THE REASONABLE COST ANALYSIS VALUE.

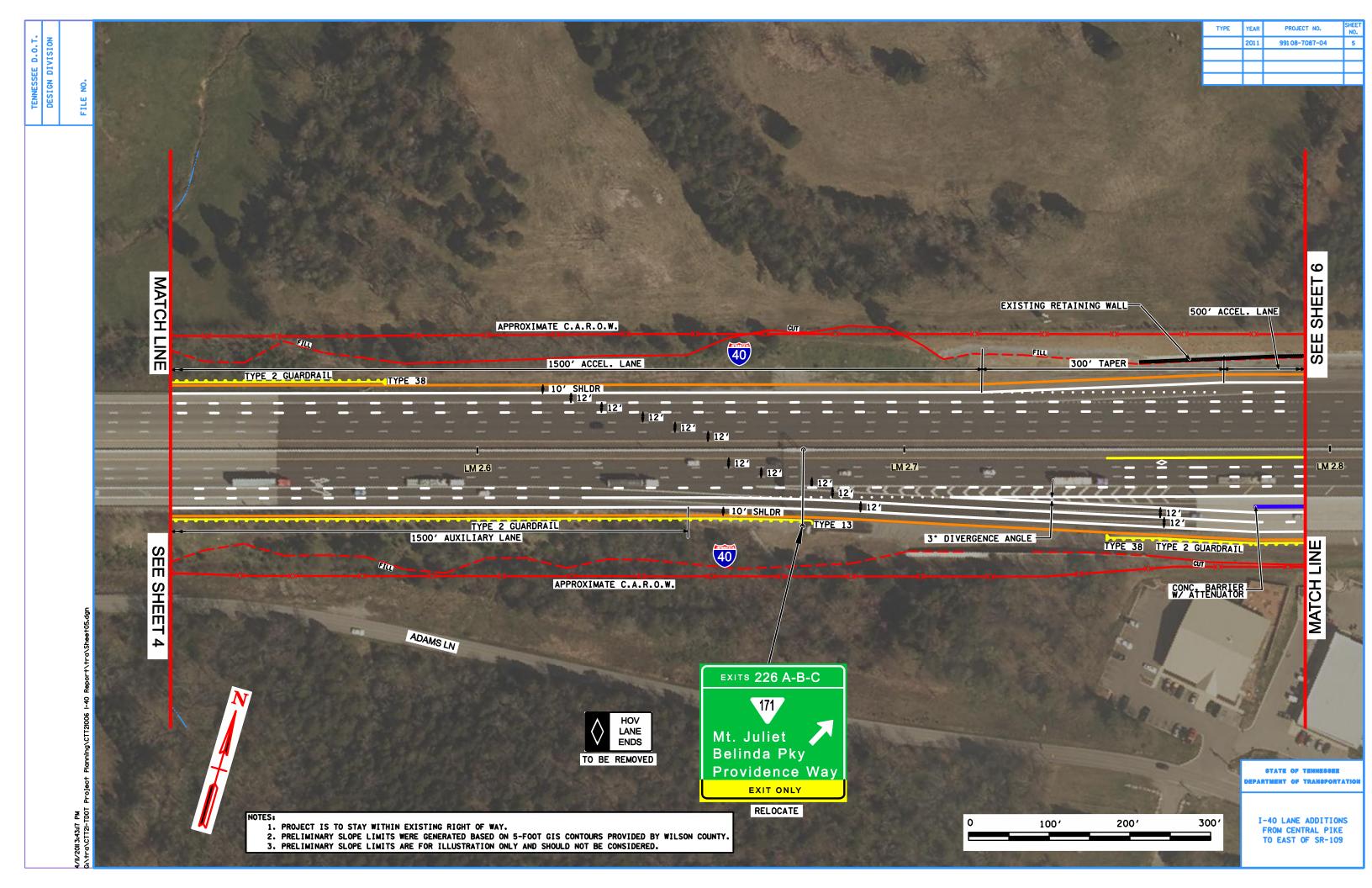
THIS PROJECT TO BE CONSTRUCTED UNDER THE STANDARD SPECIFICATIONS OF THE TENNESSEE DEPARTMENT OF TRANSPORTATION DATED MARCH 1, 2006 AND ADDITIONAL SPECIFICATIONS AND SPECIAL PROVISIONS CONTAINED IN THE PLANS AND IN THE PROPOSAL CONTRACT.

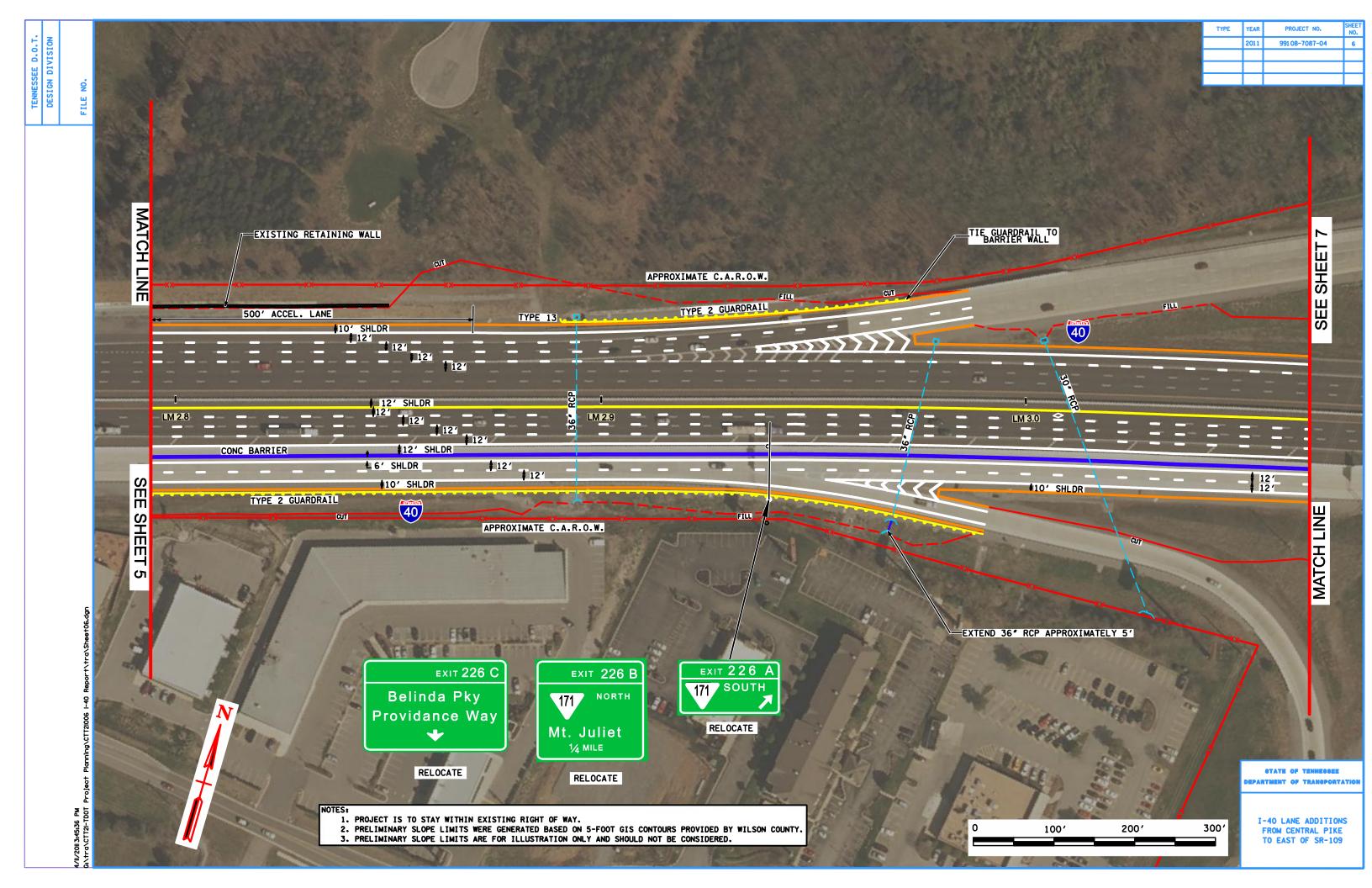
TDOT C.E. MANAGER 1 OR TDOT DESIGN MANAGER 1 TDOT ROAD SP. SV. 2 DESIGNED BY	
DESIGNER	
P.E. NO	

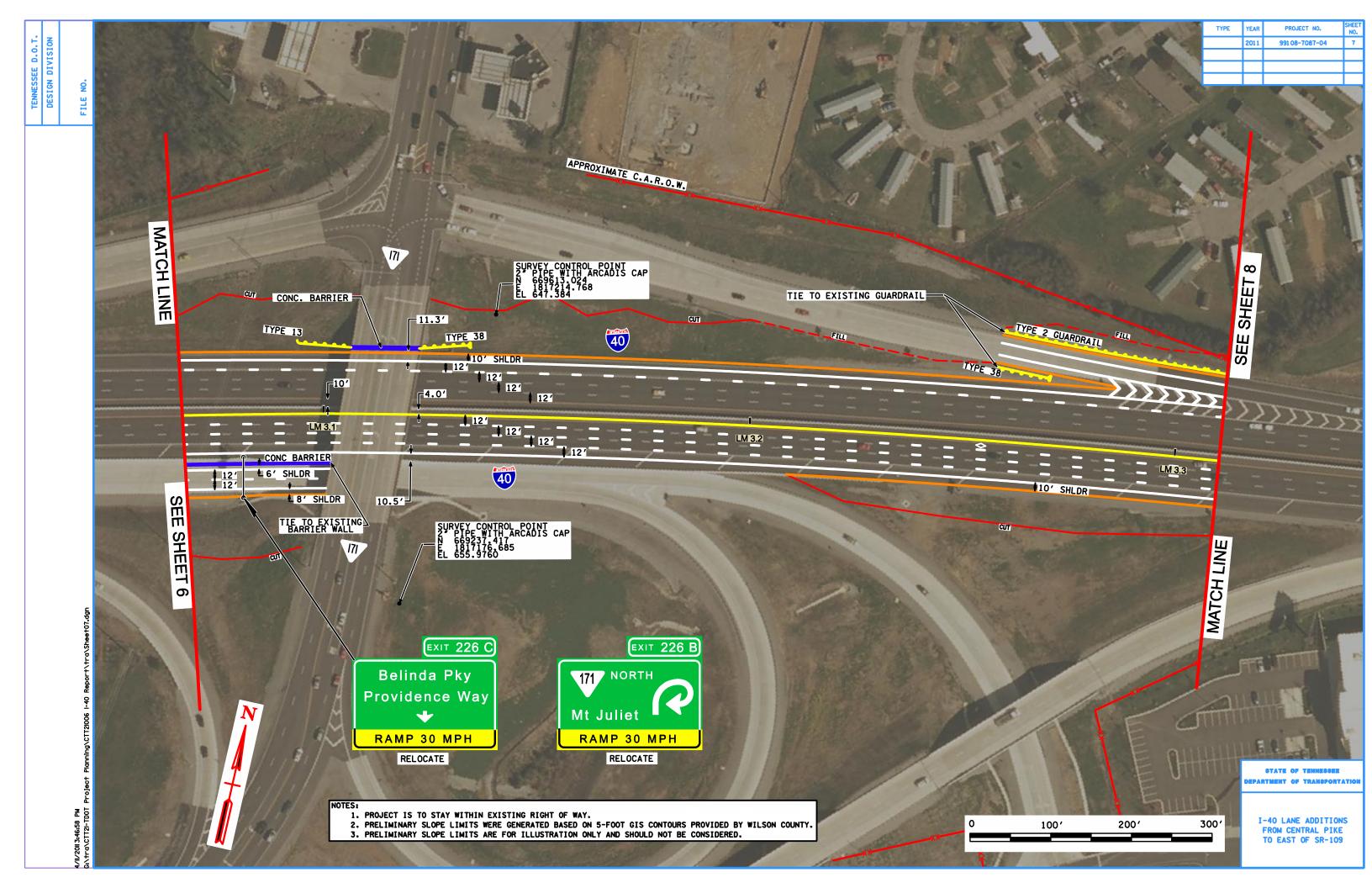


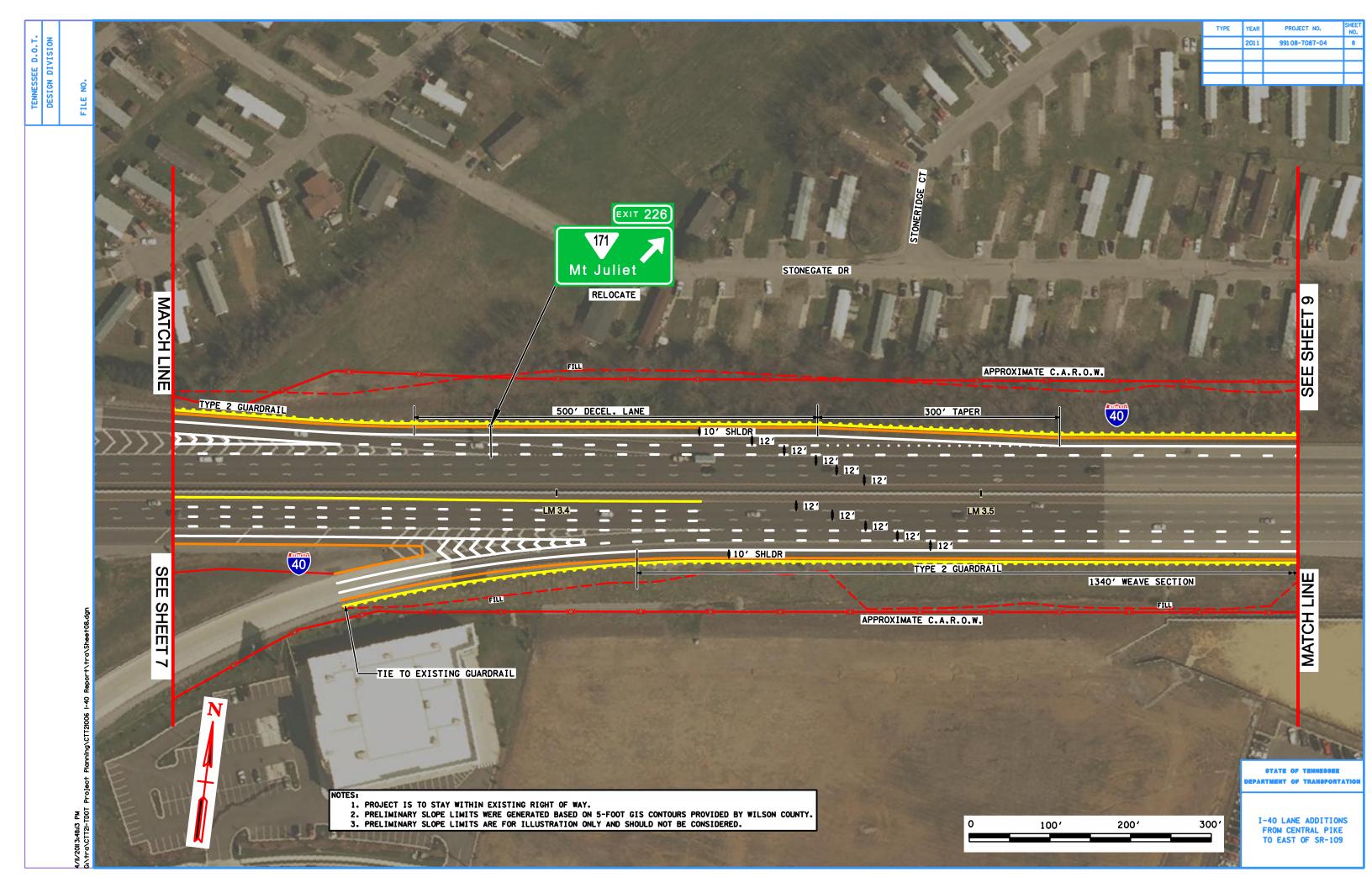


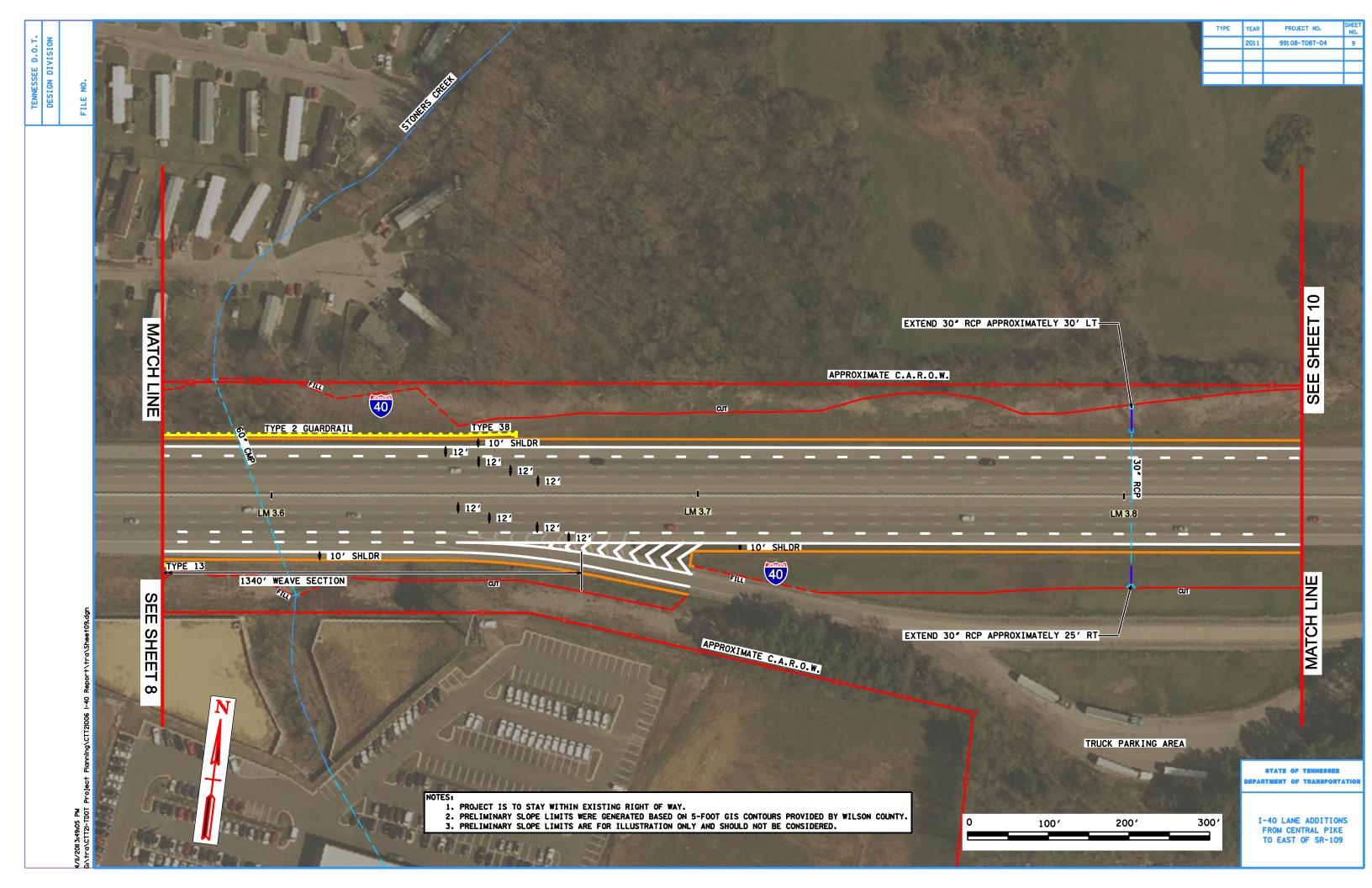


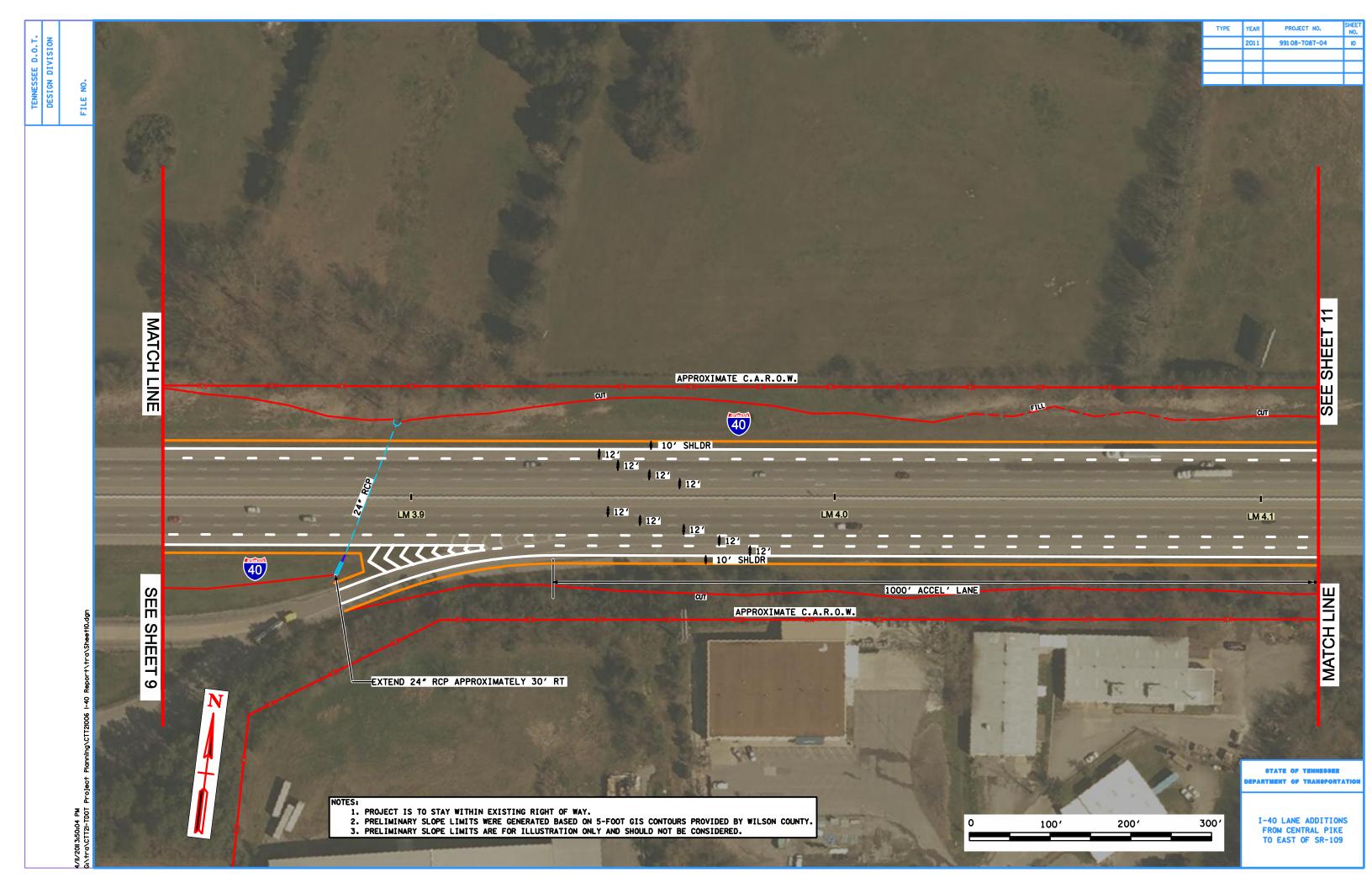


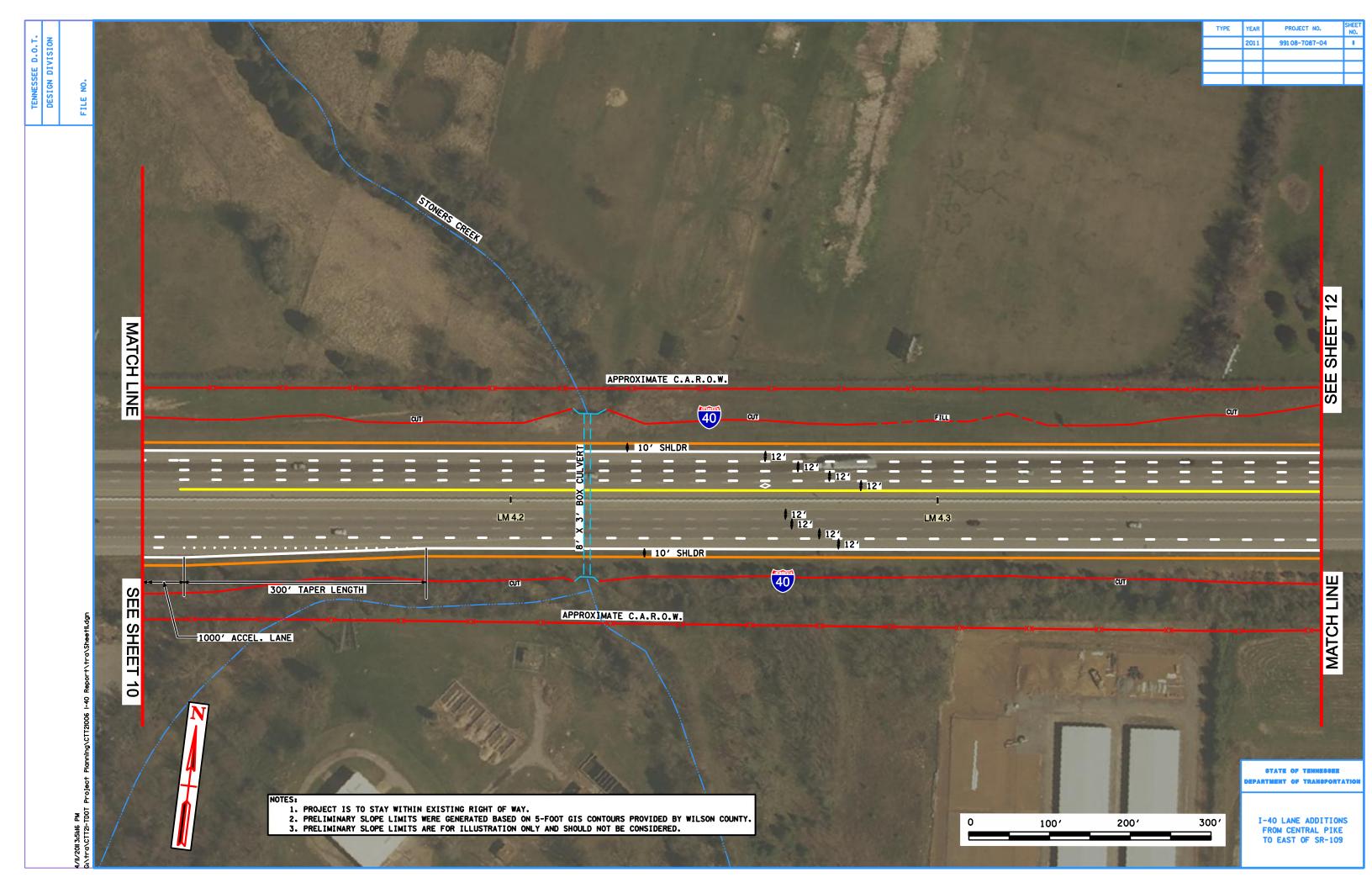


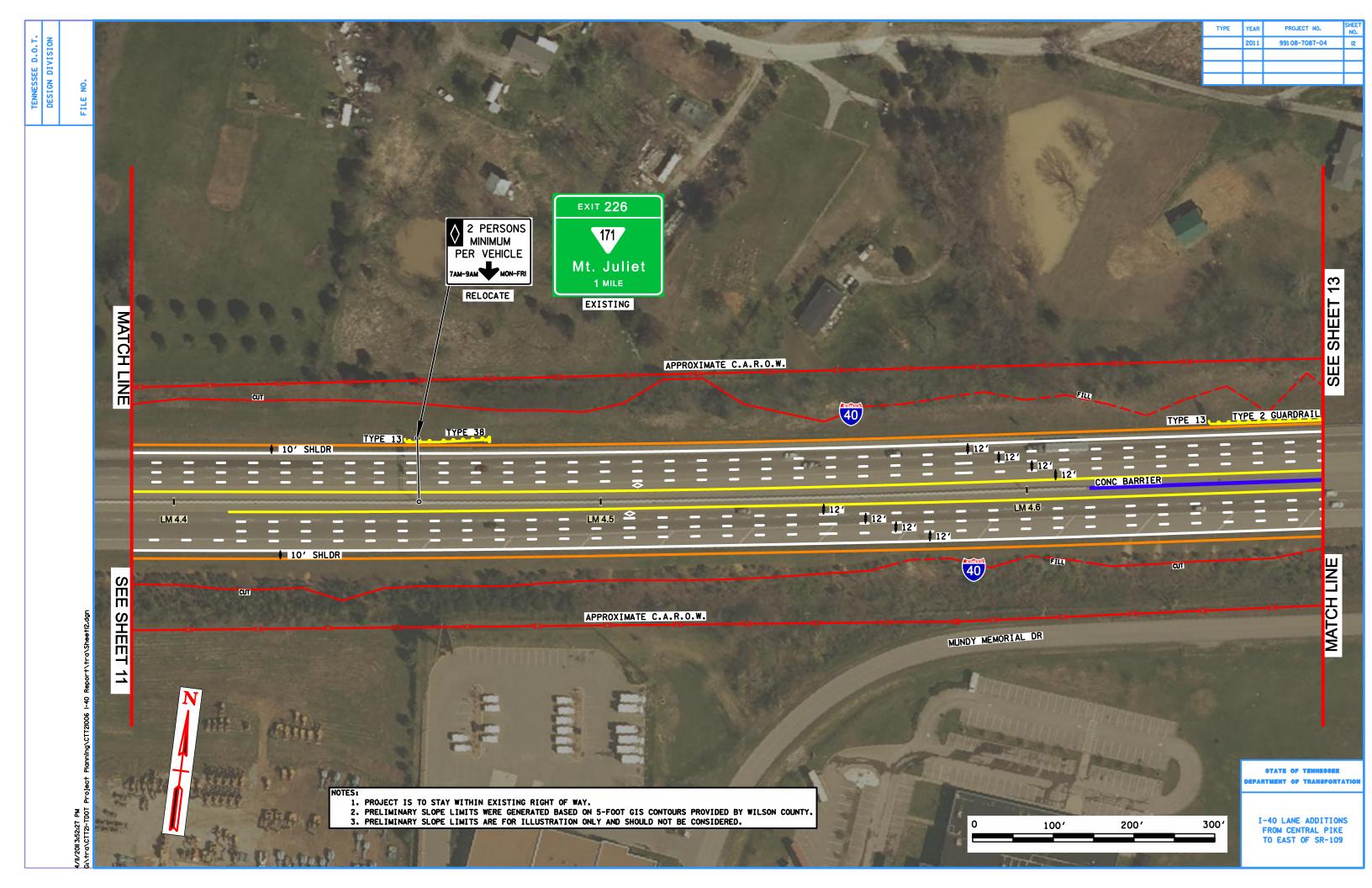


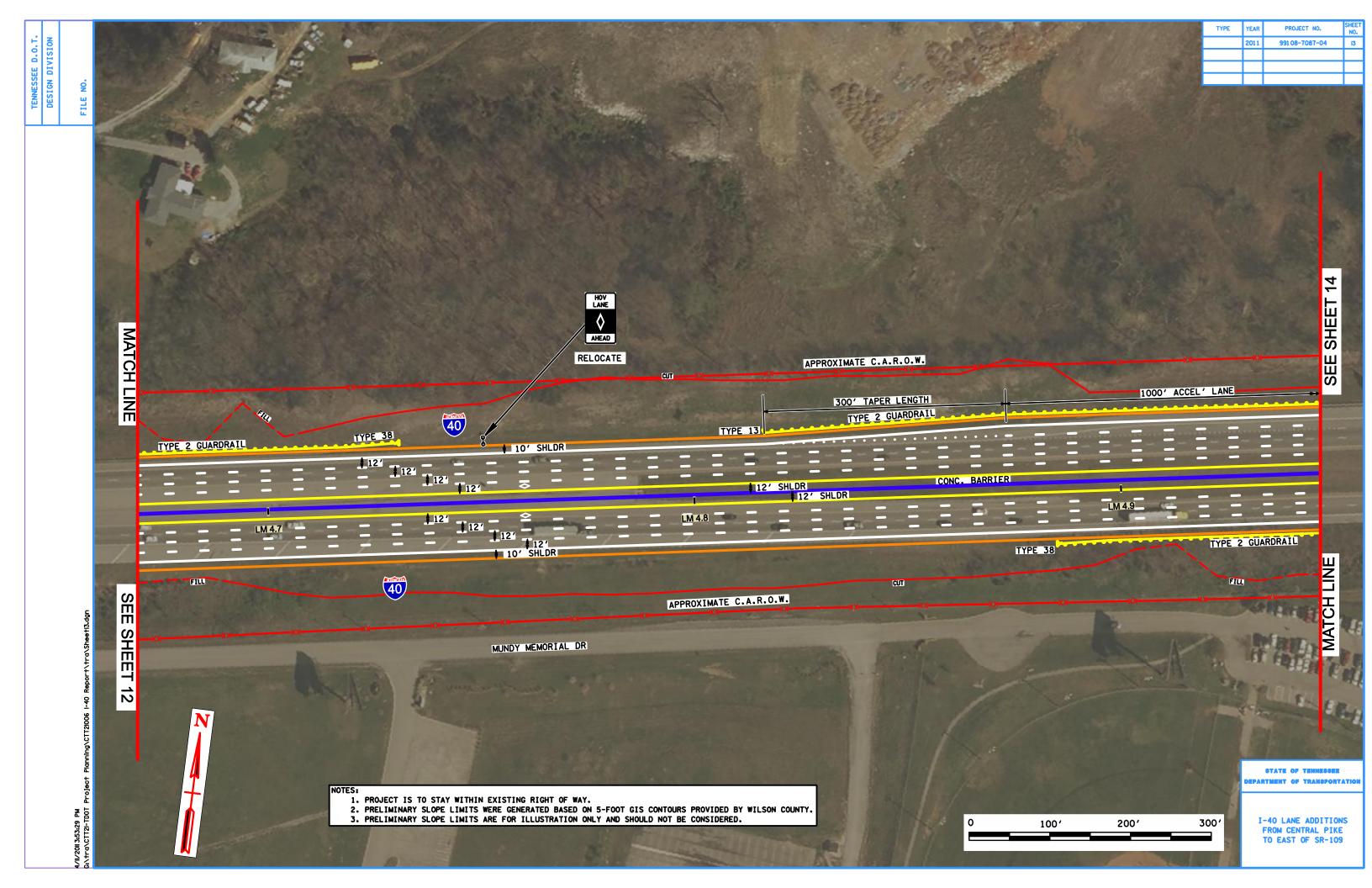


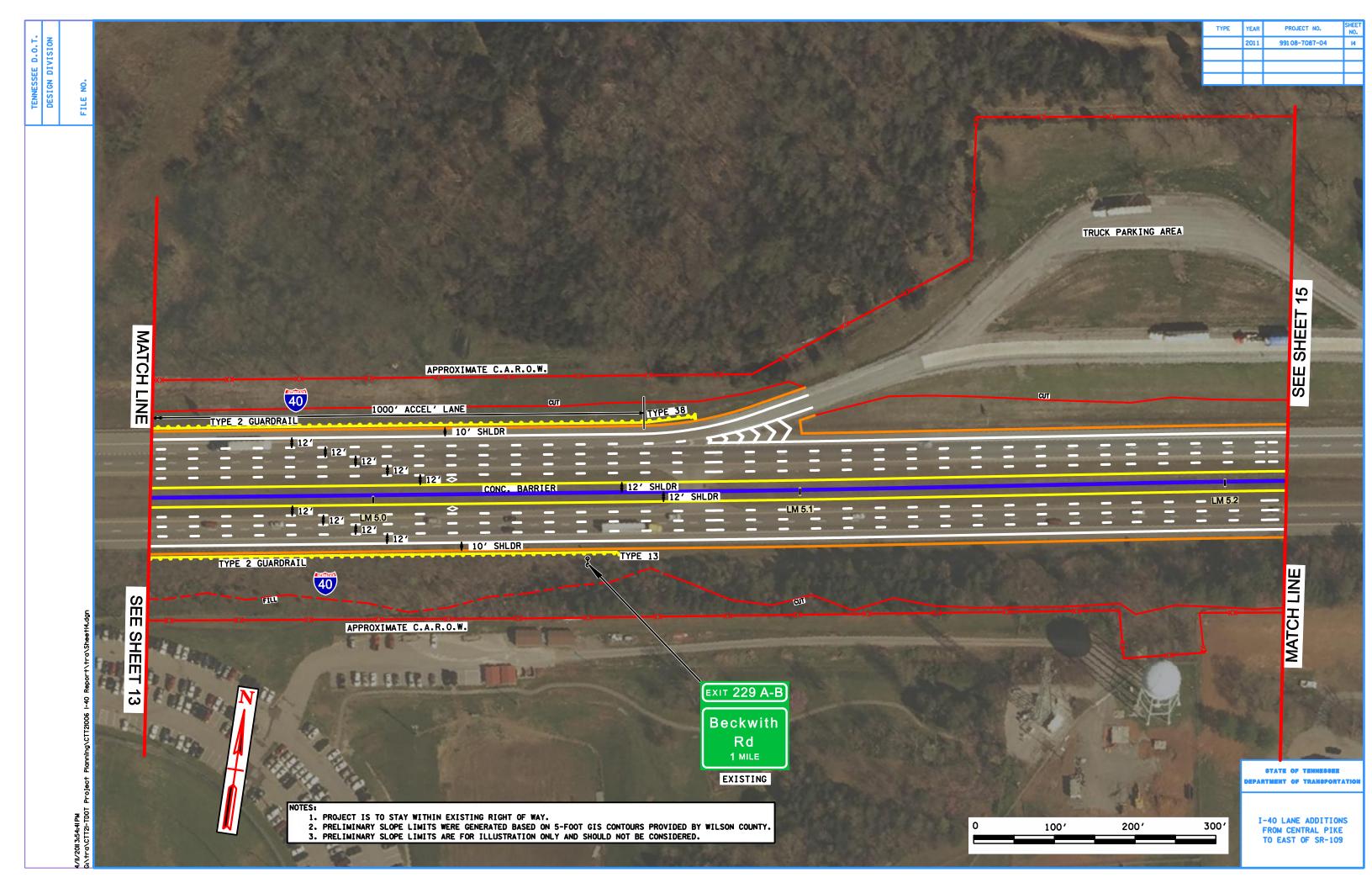


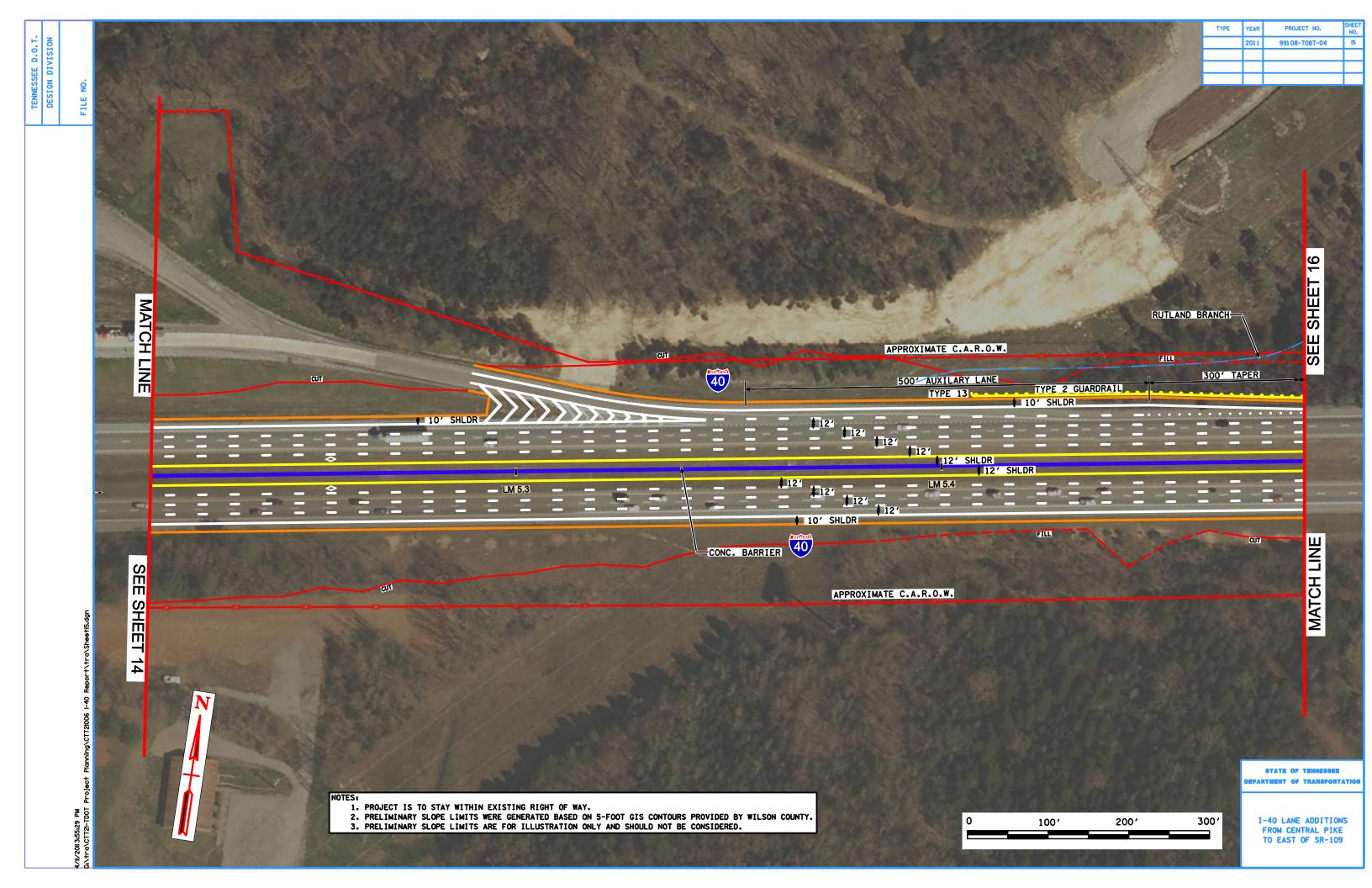


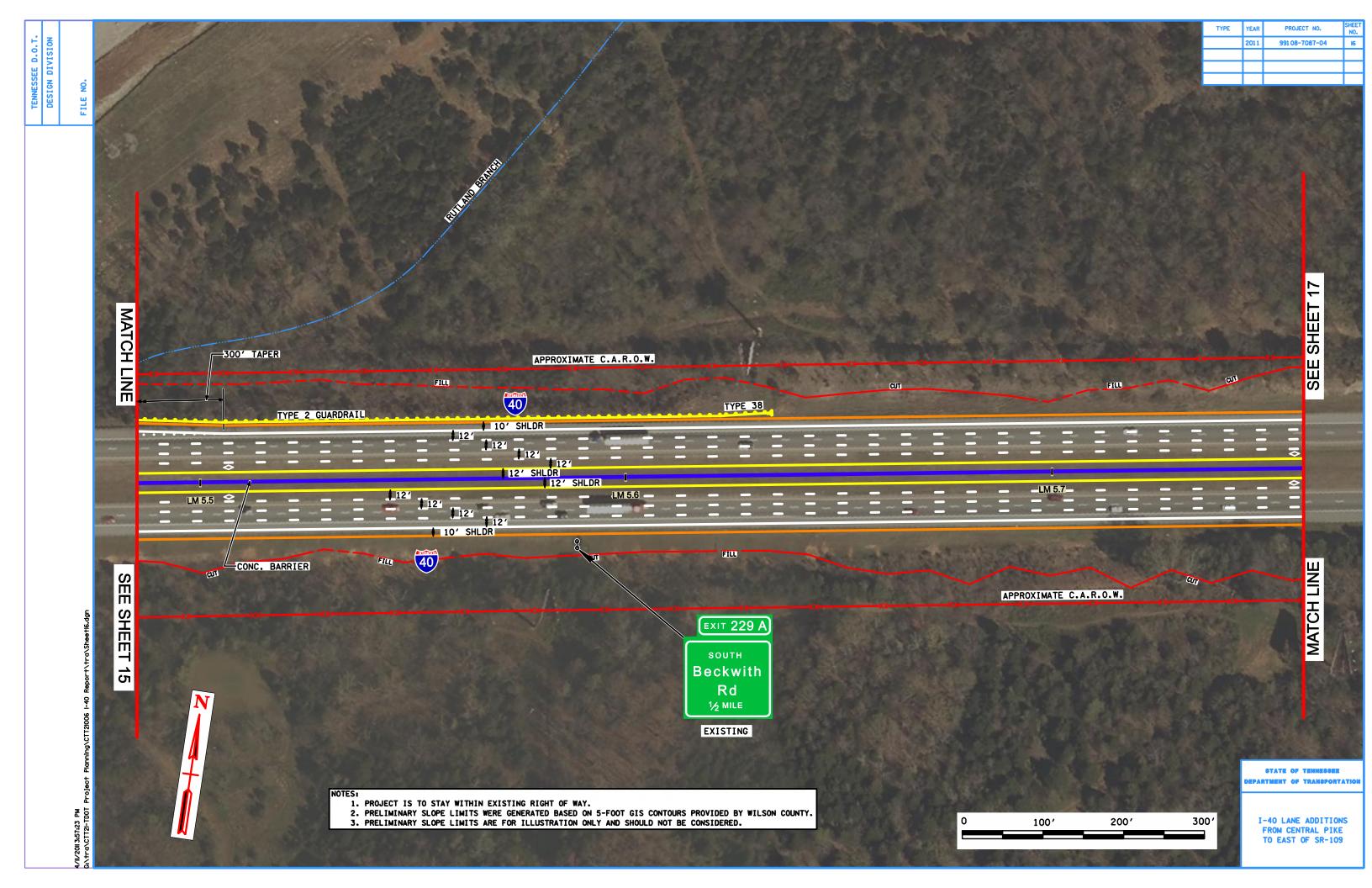


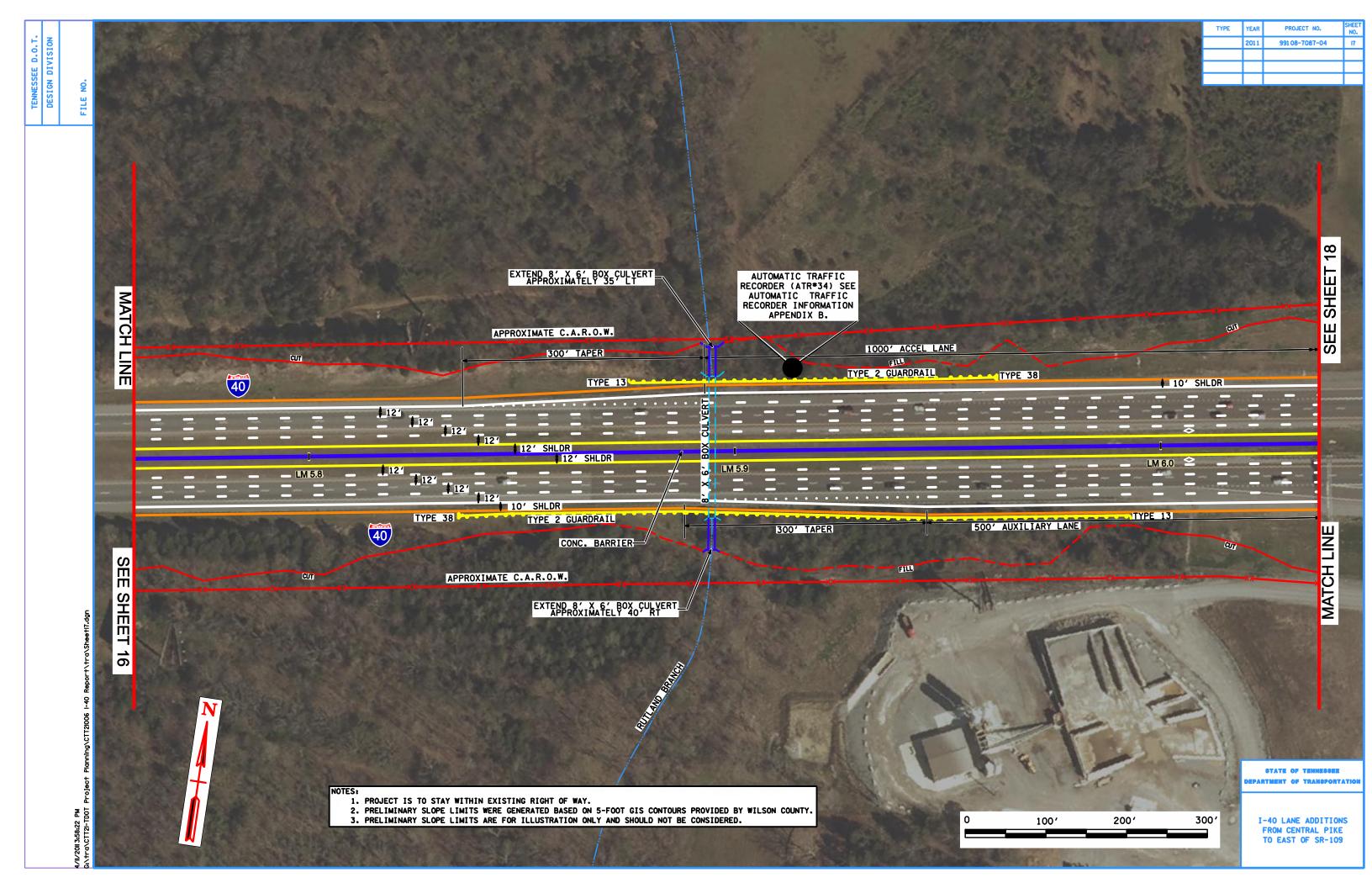


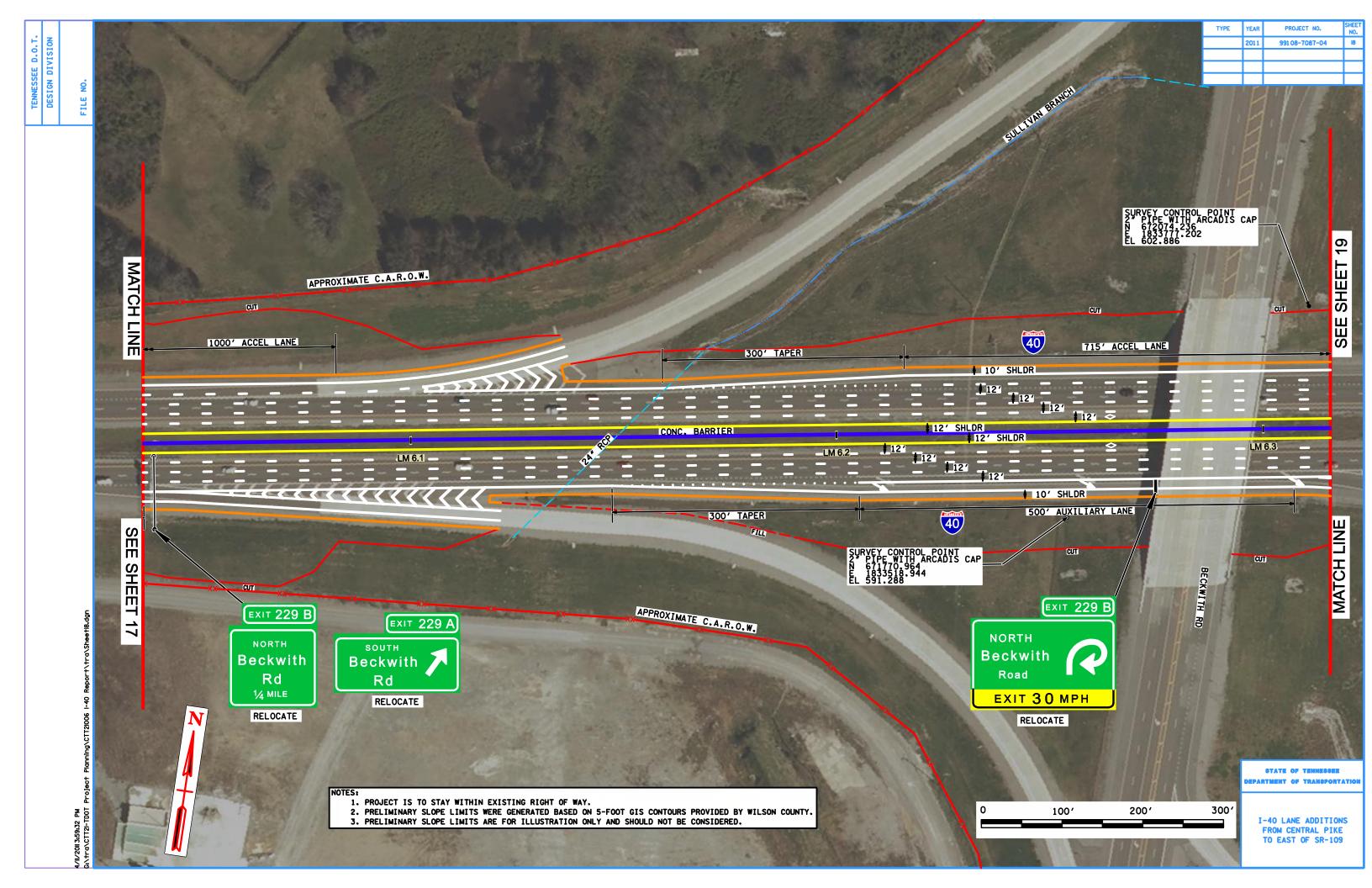


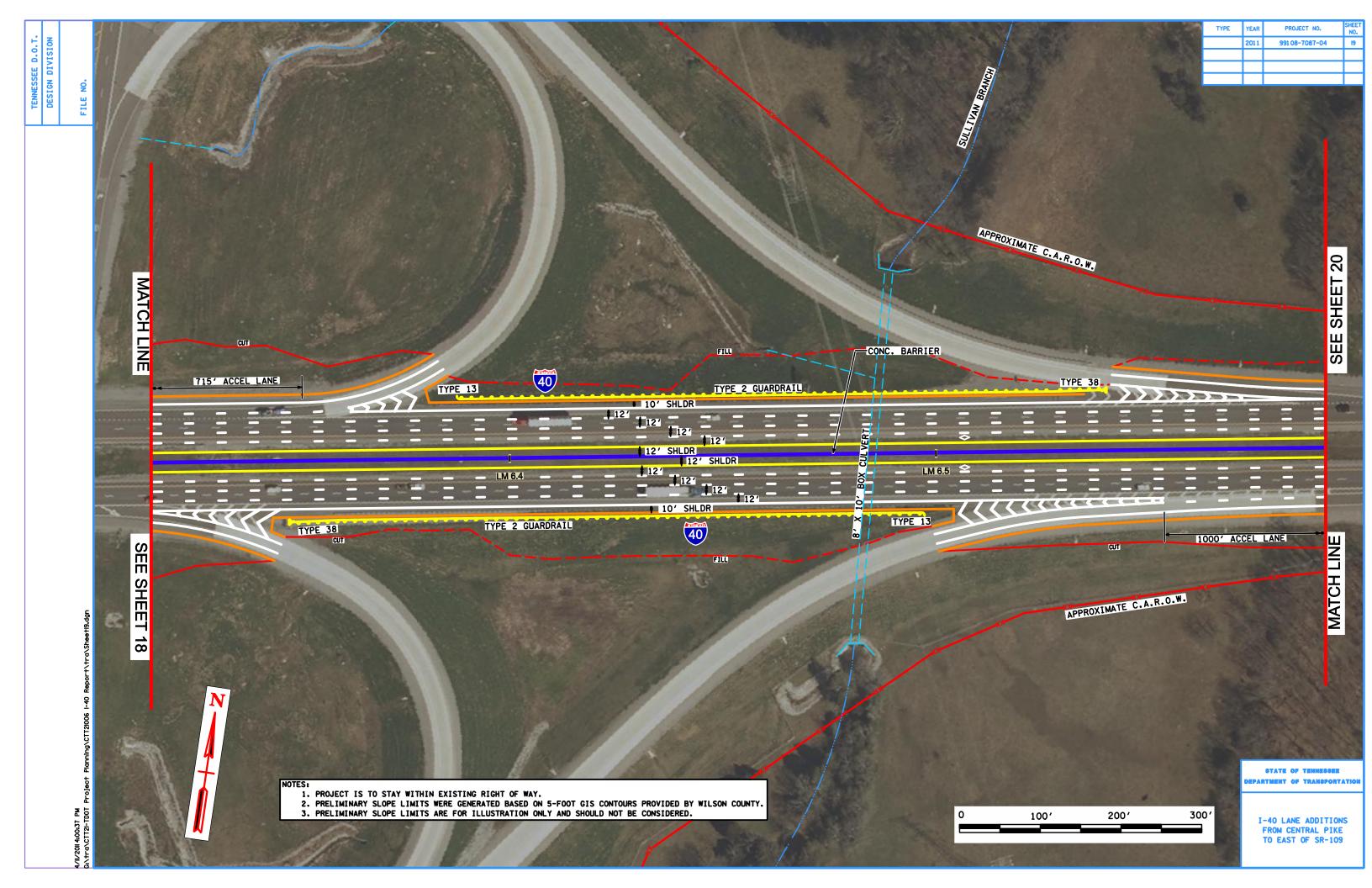


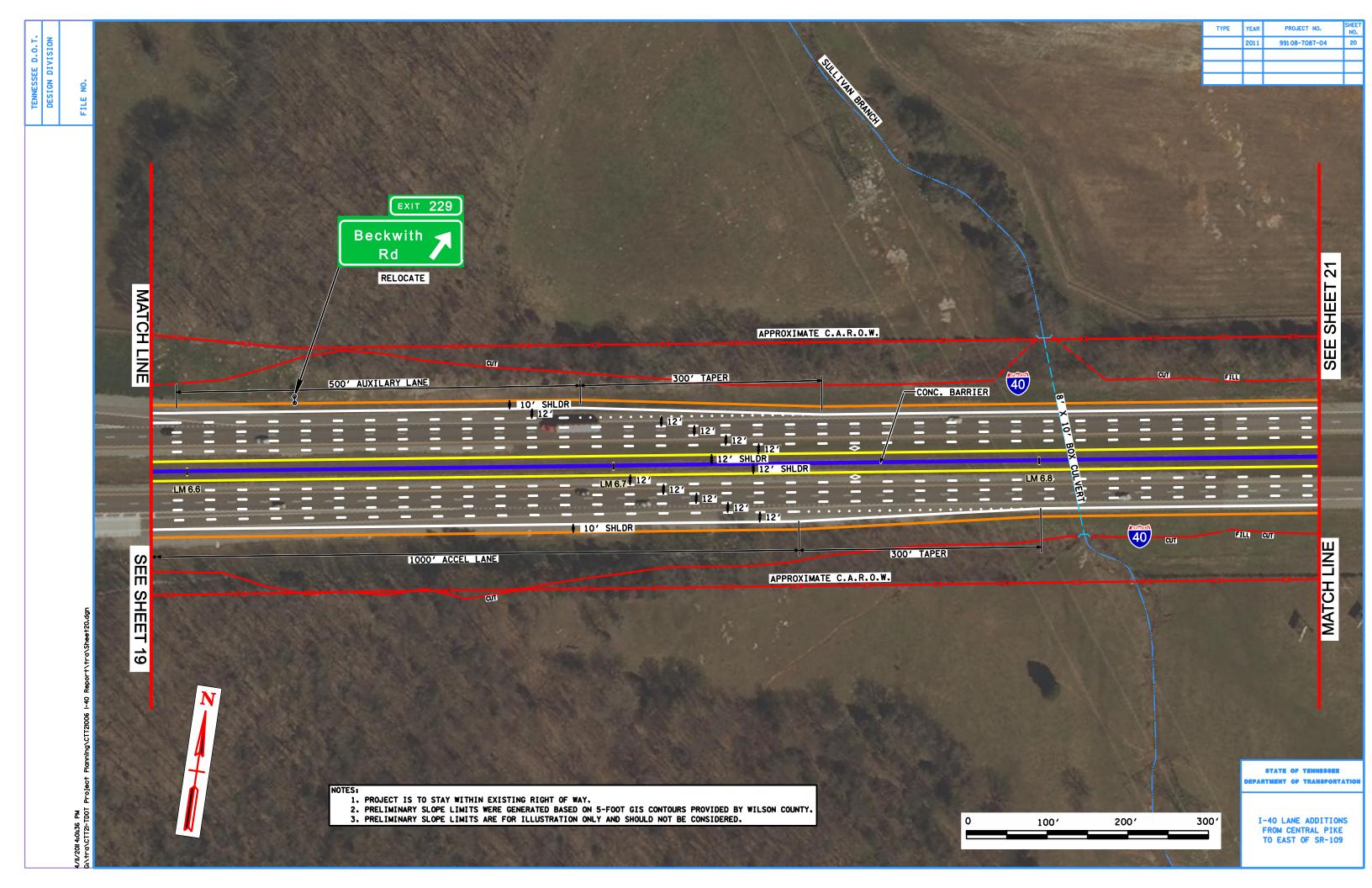


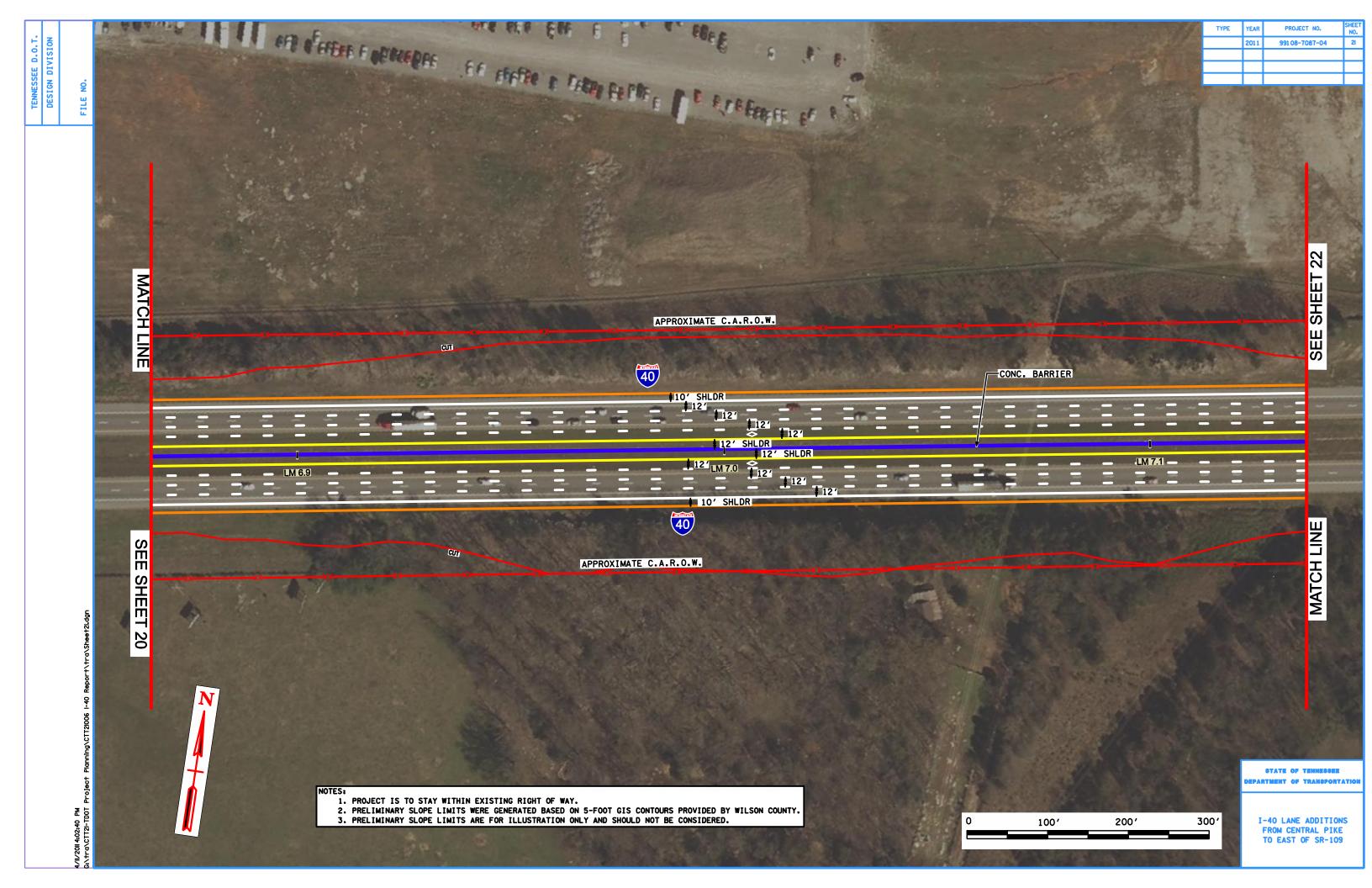


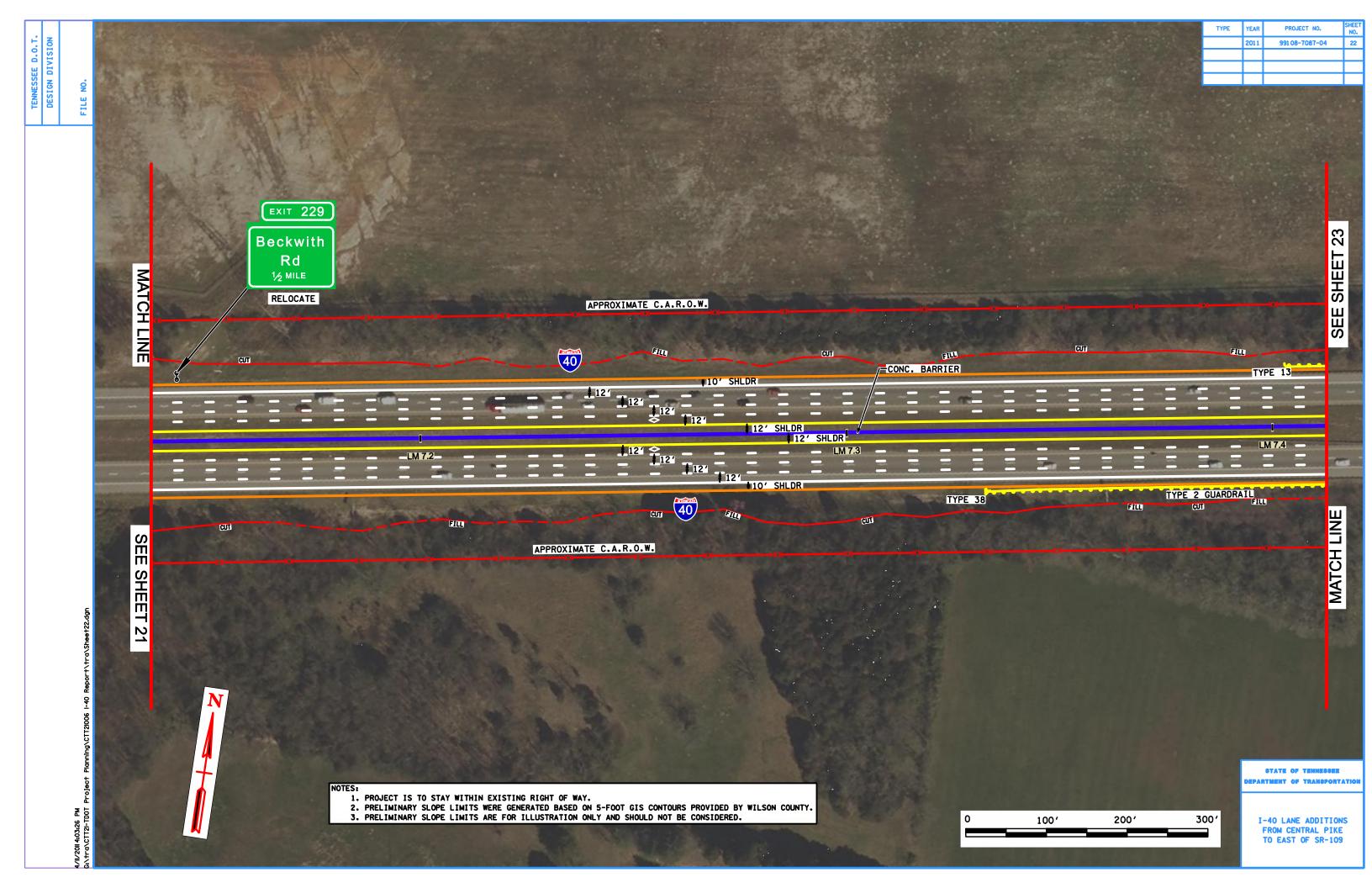


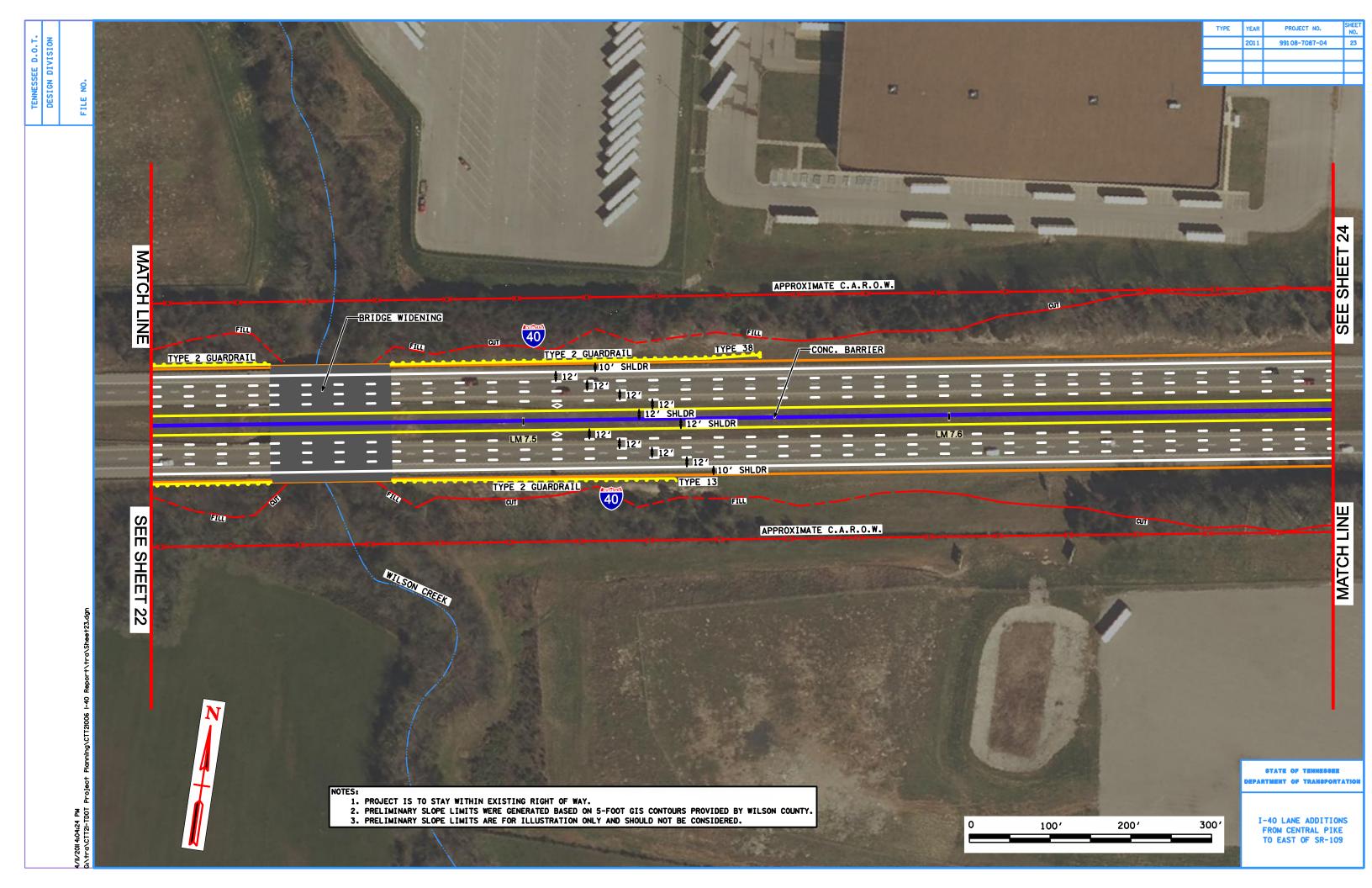


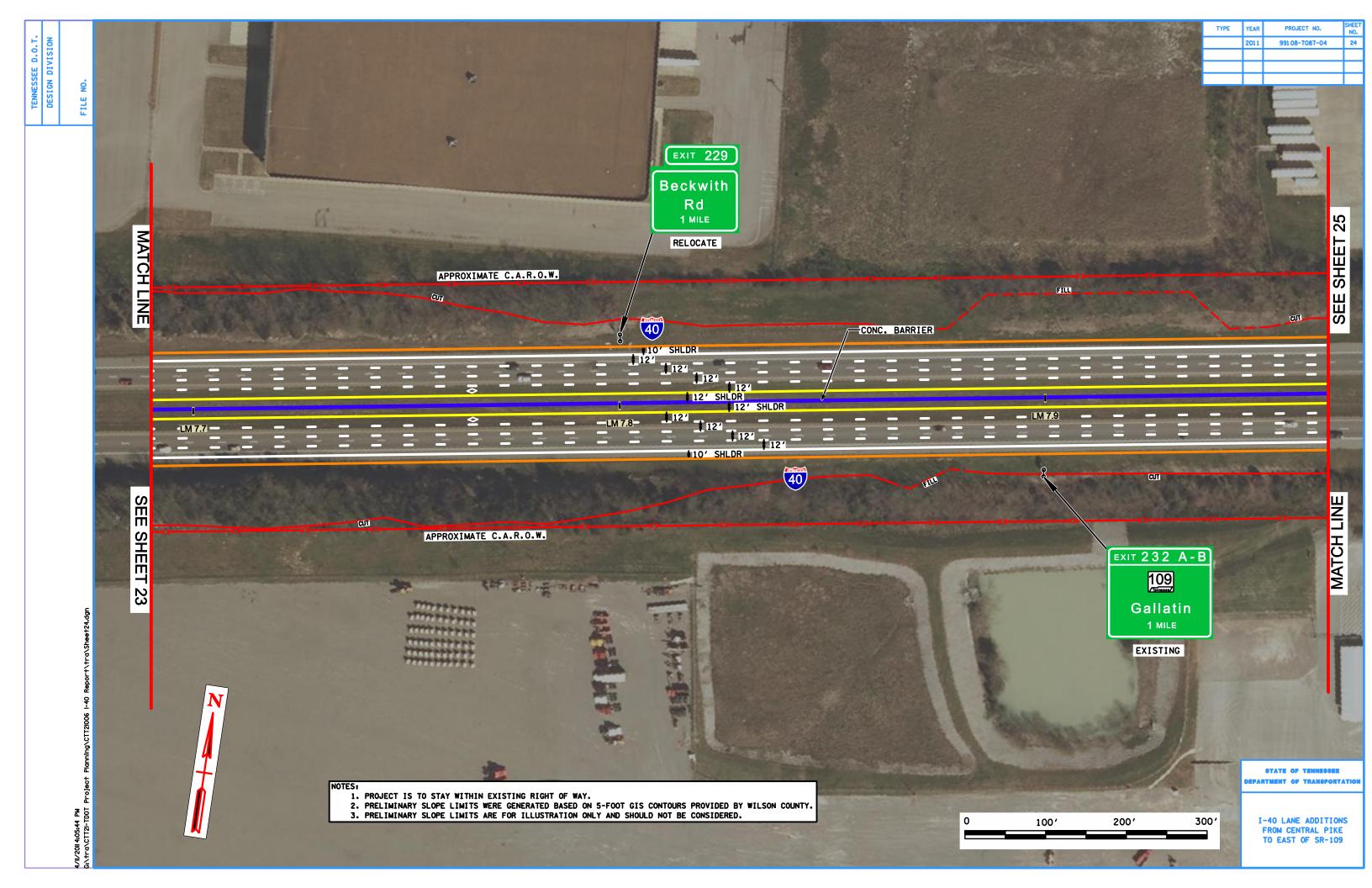


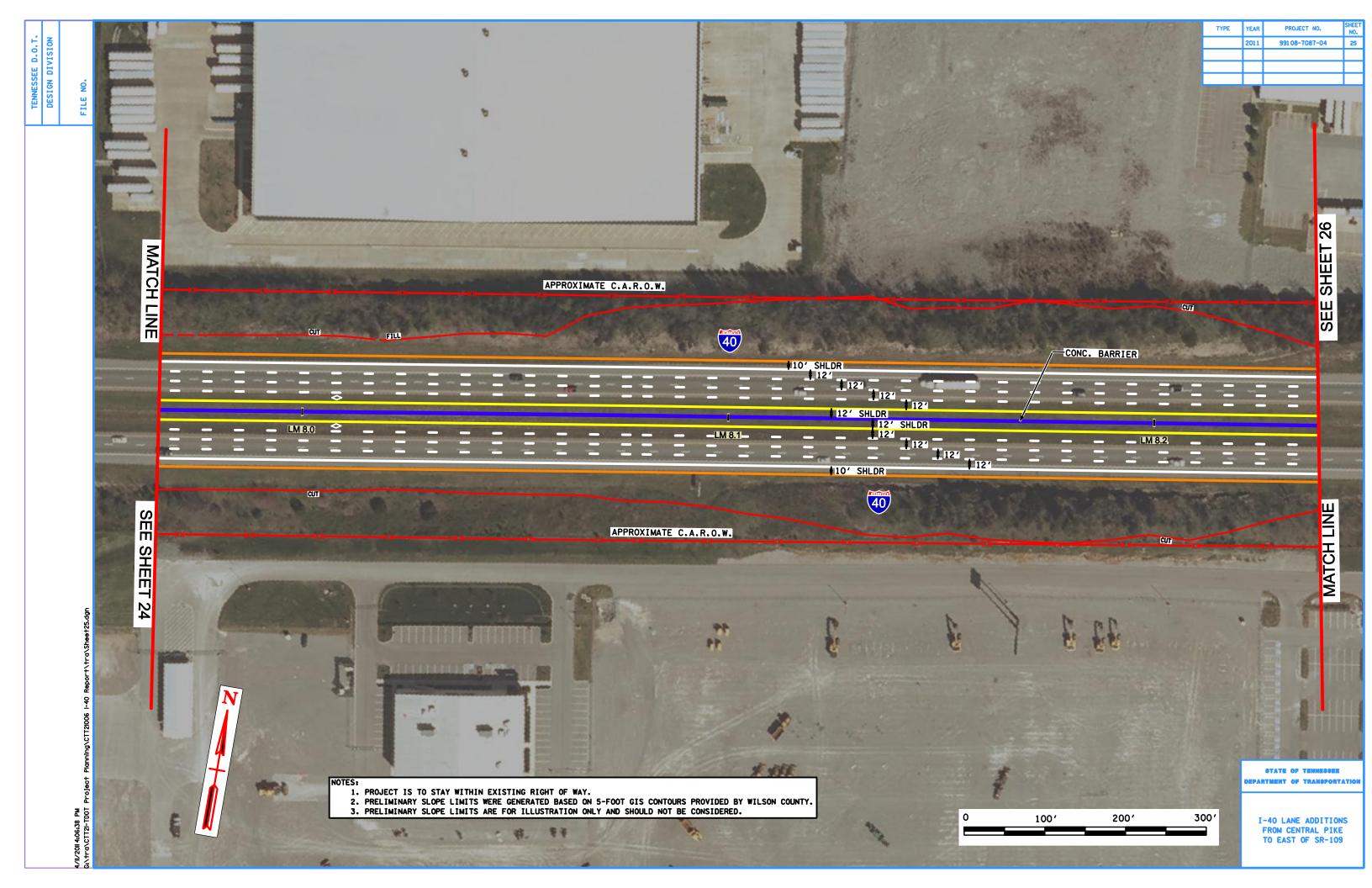


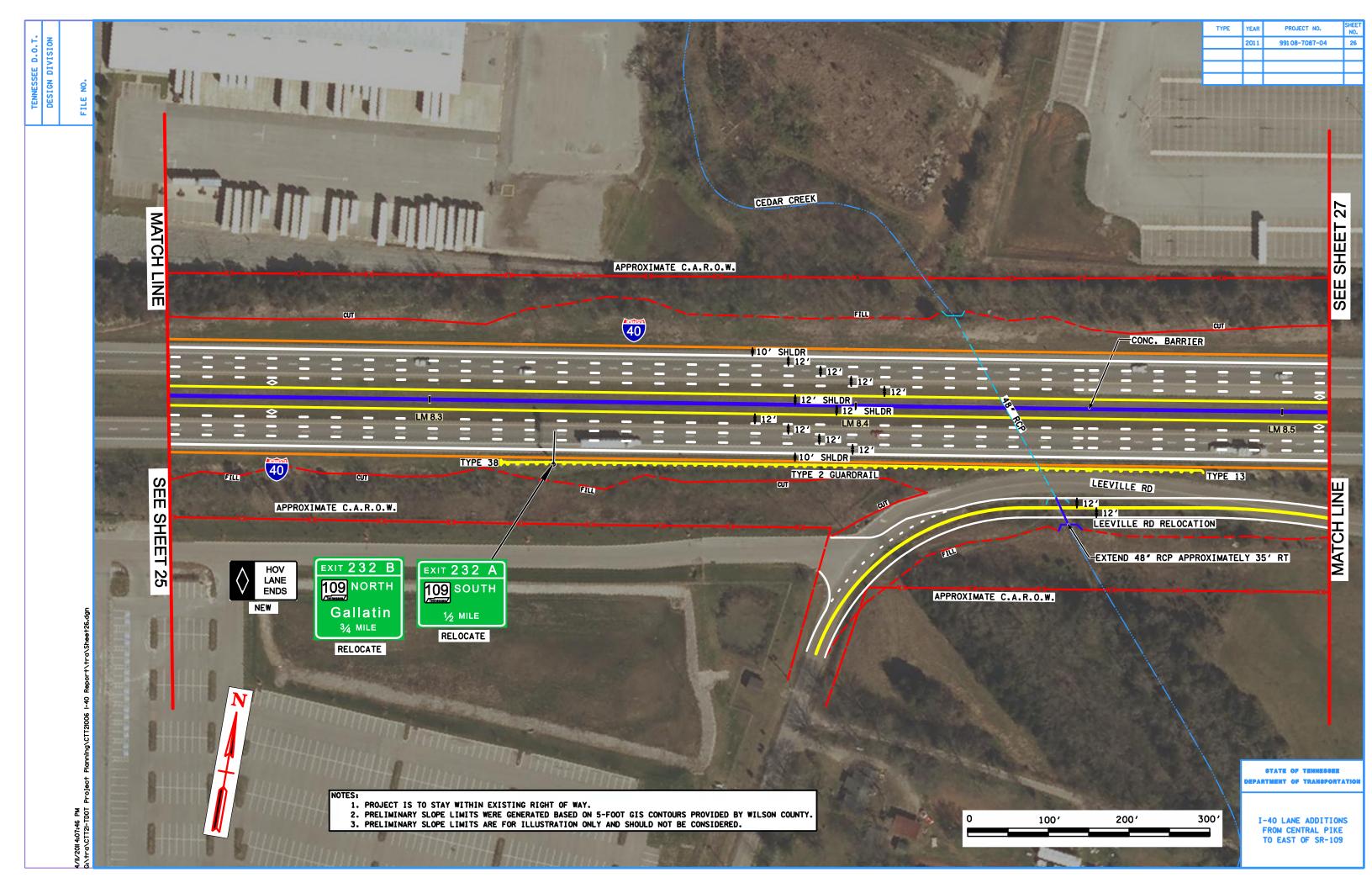


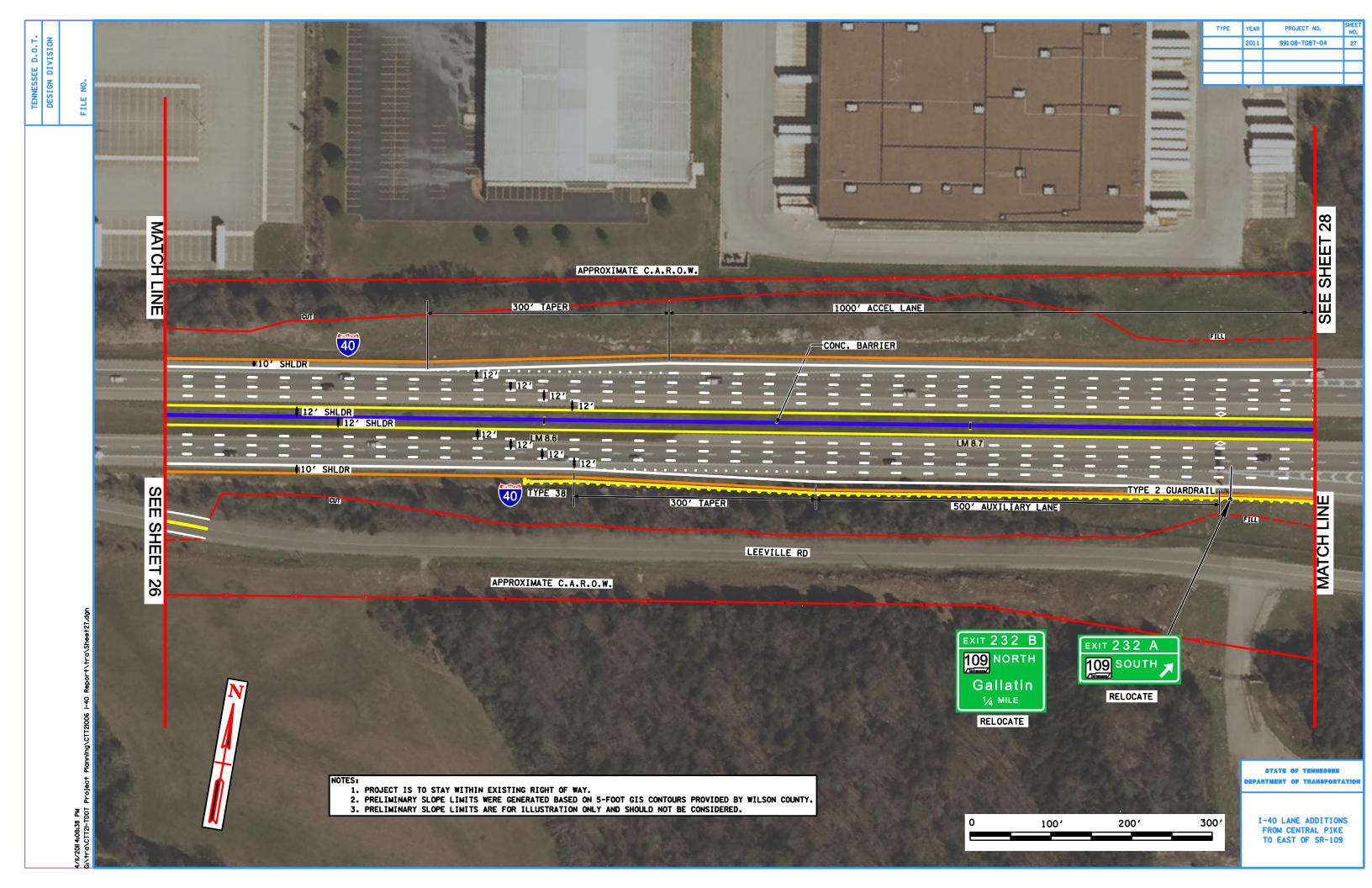


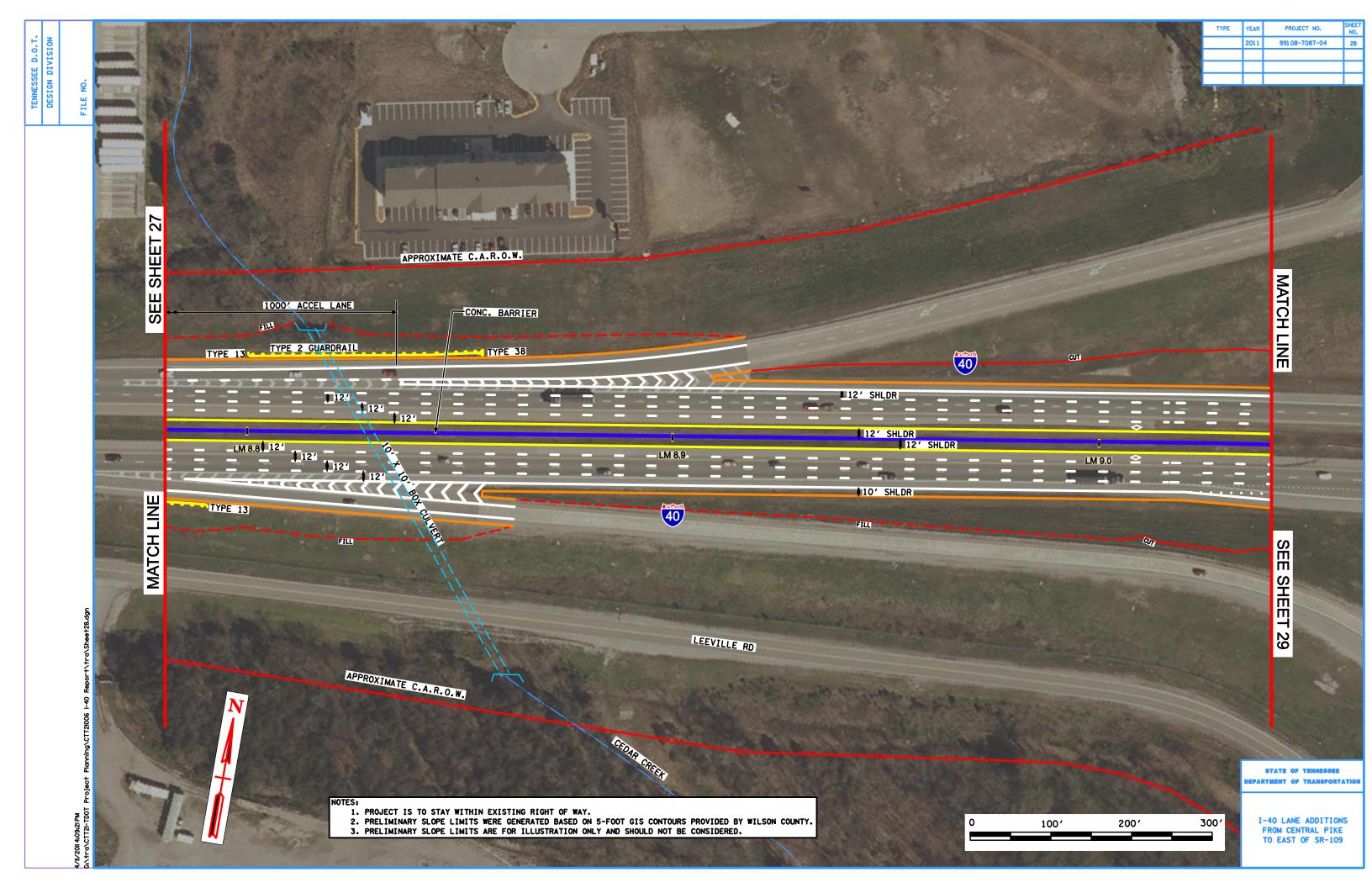


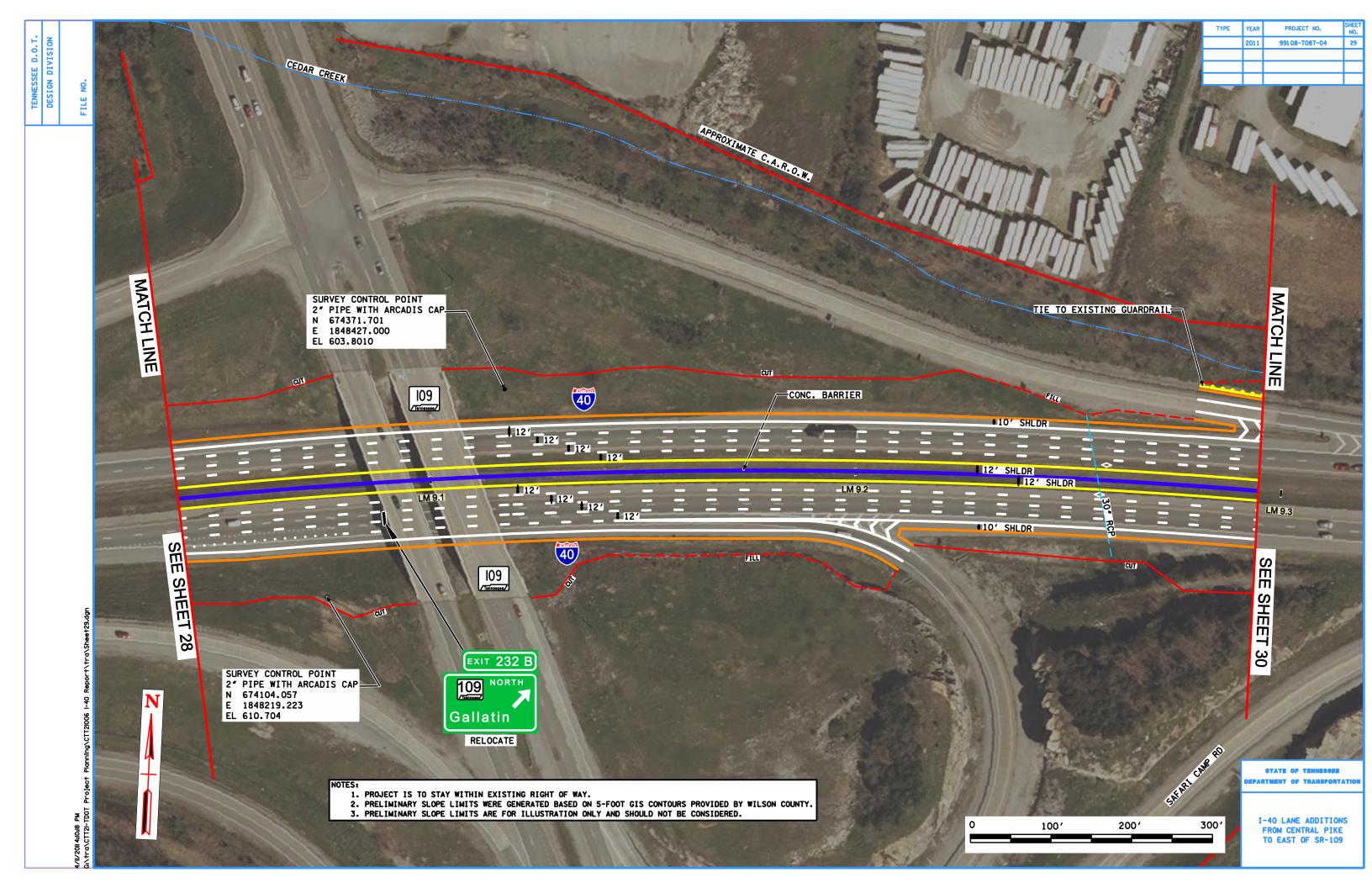


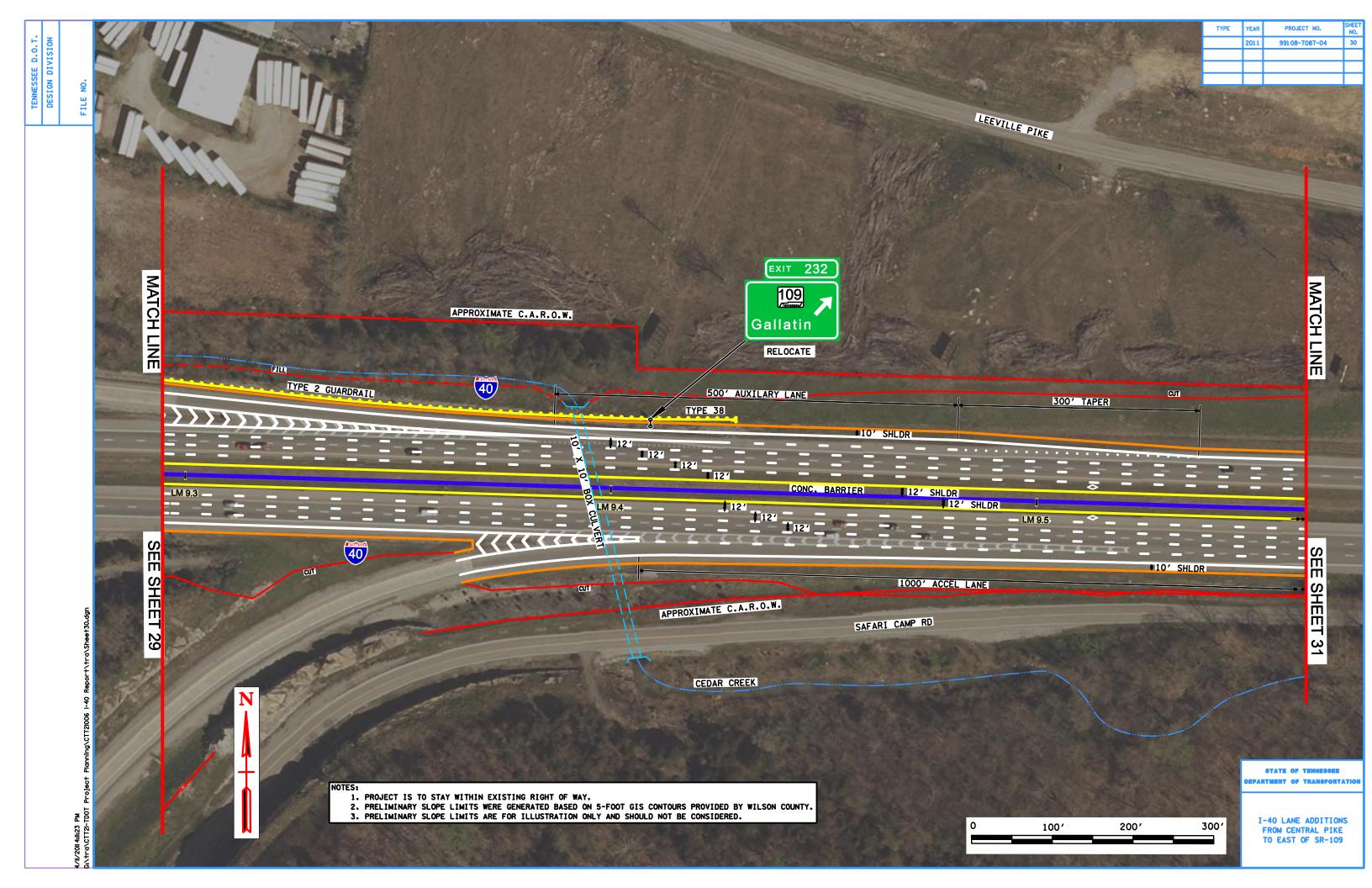


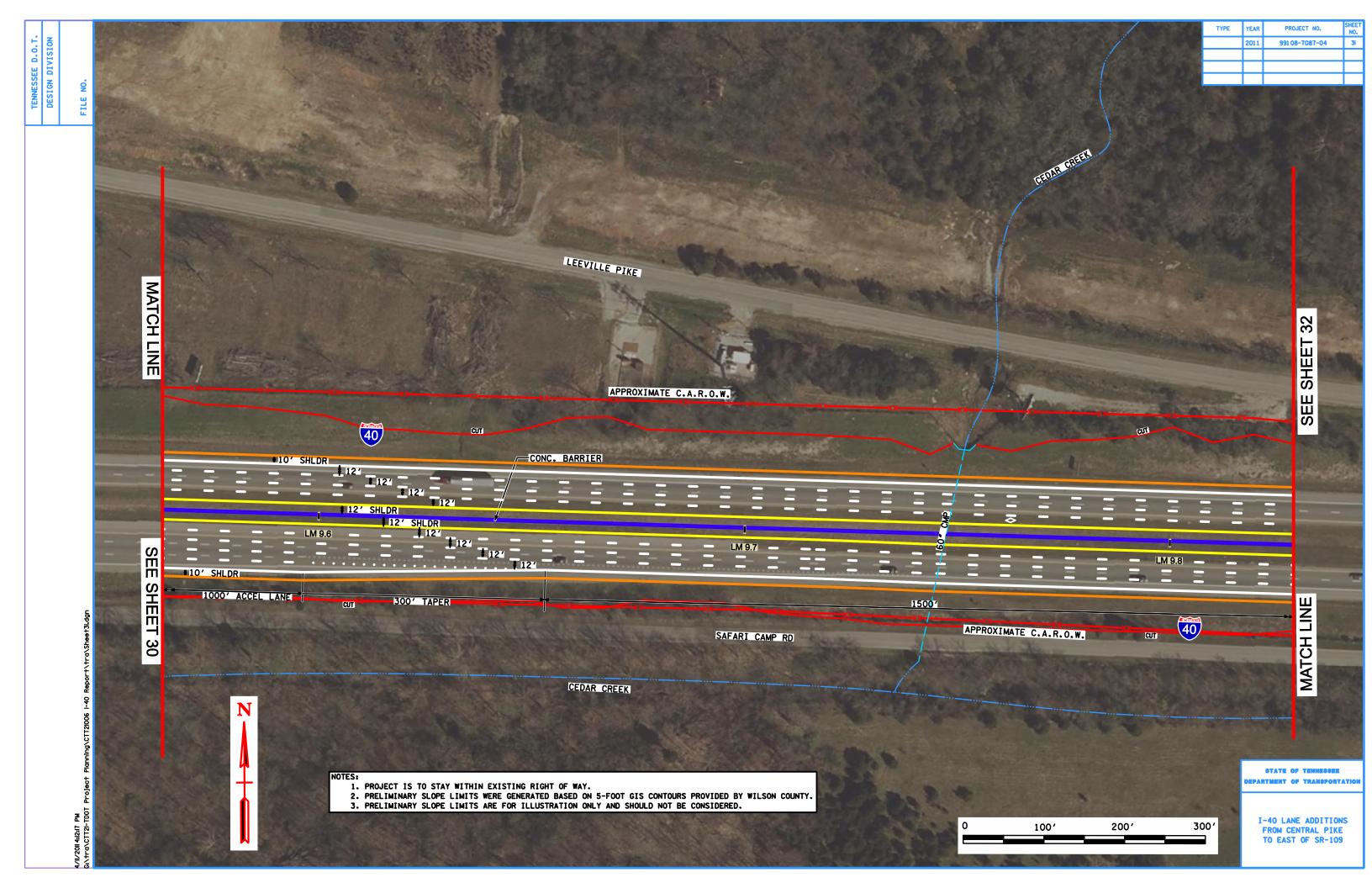


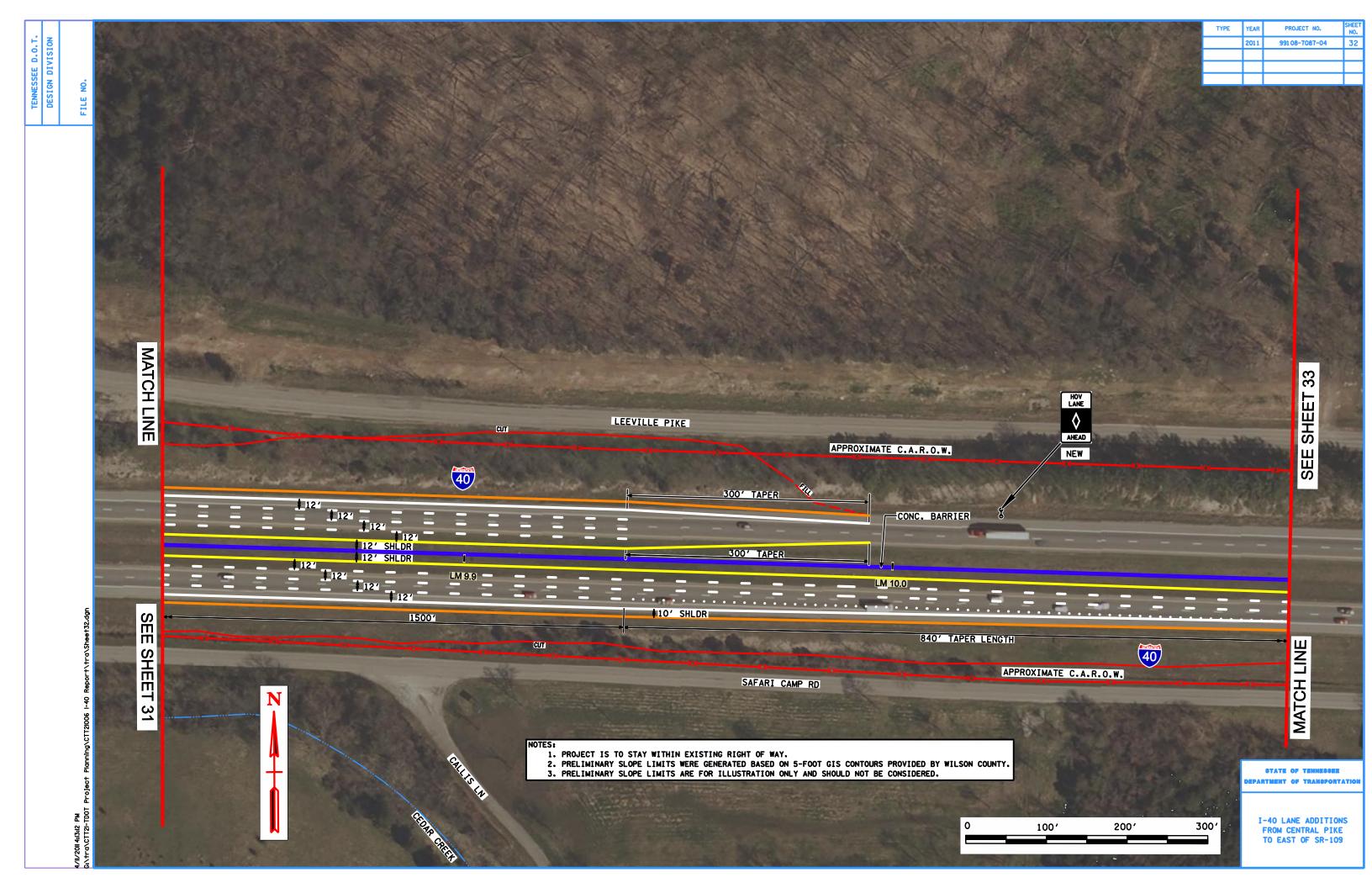


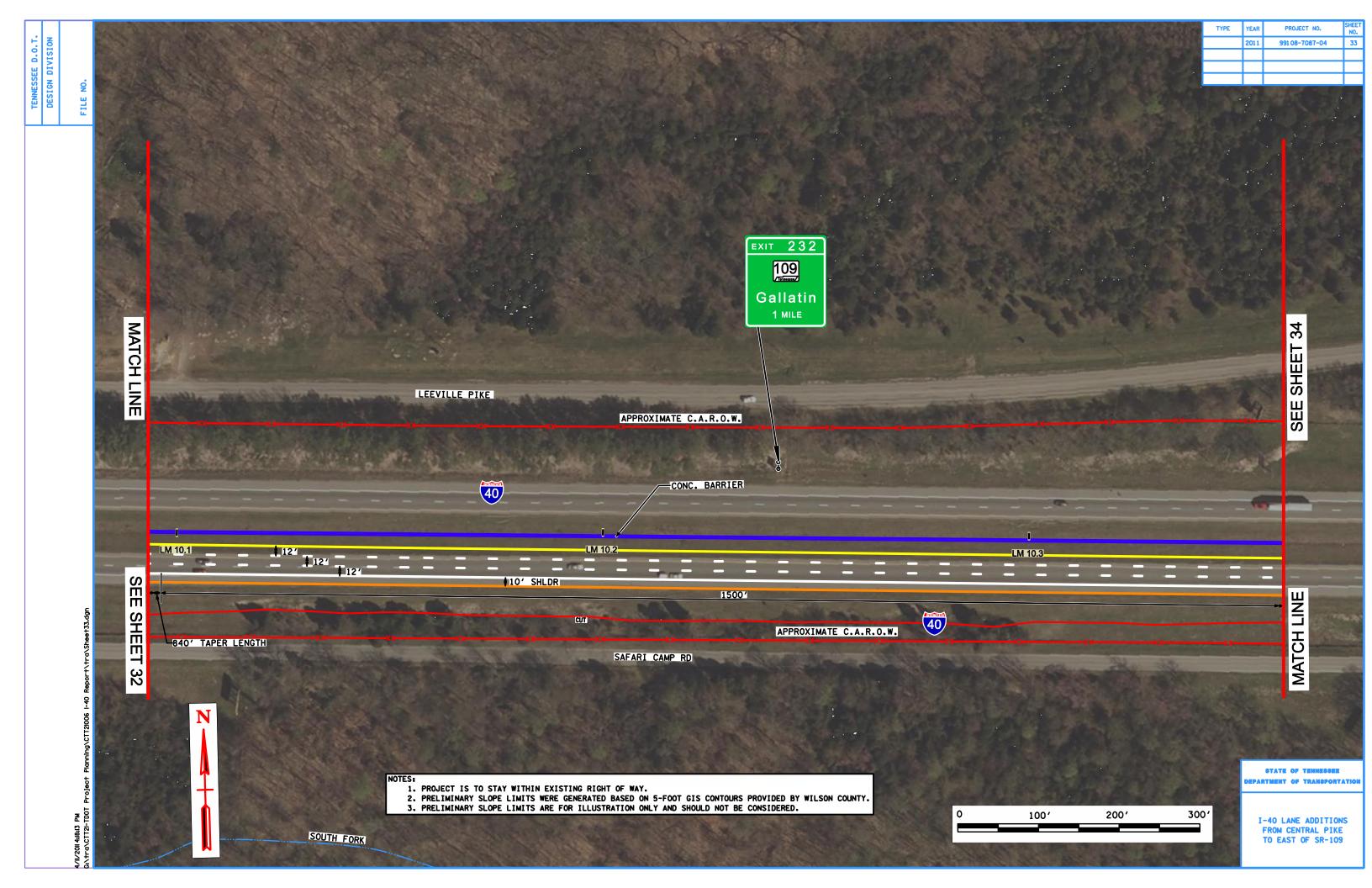


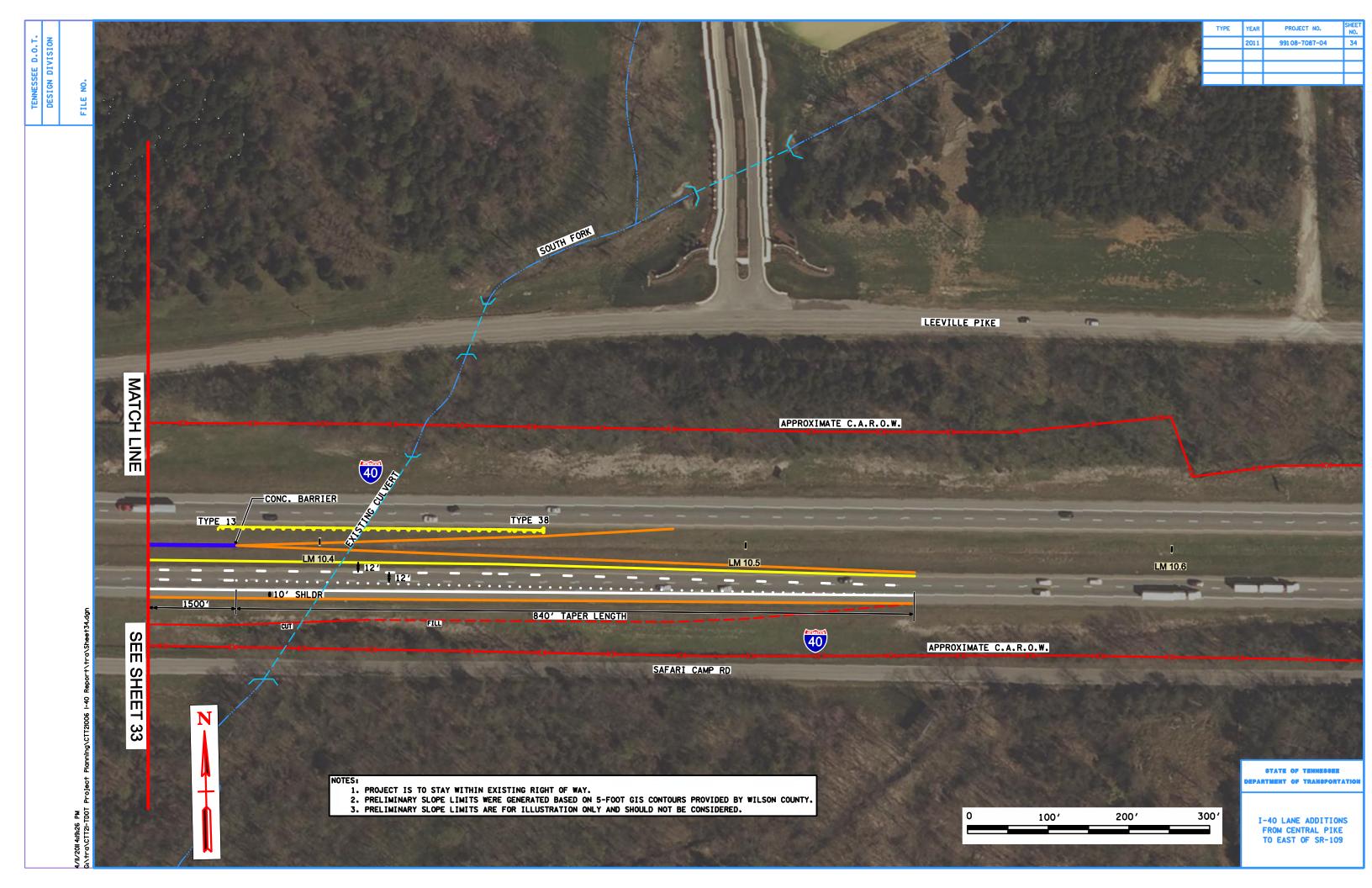












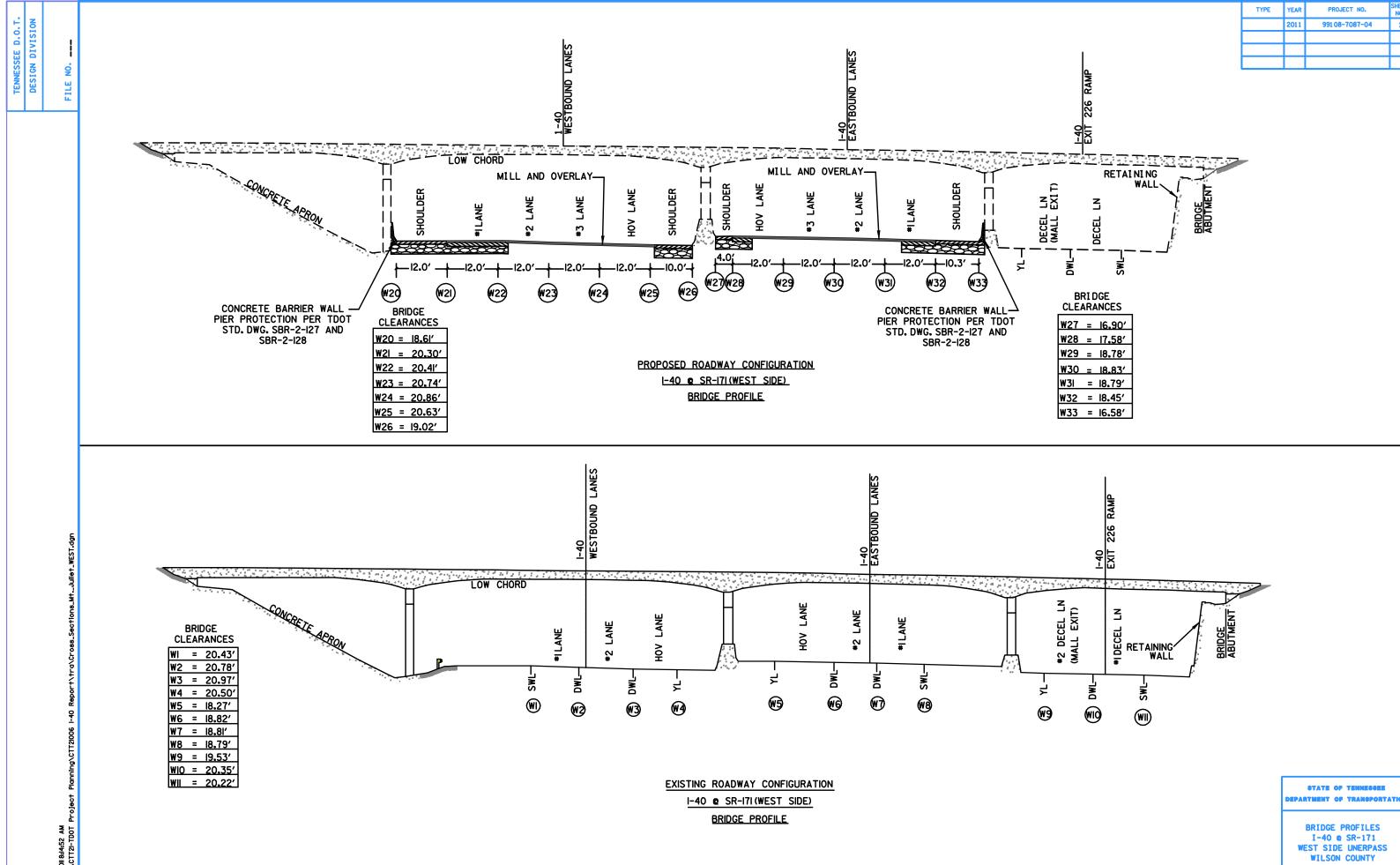
Project Description: Interstate 40 Lane Additions

Technical Report

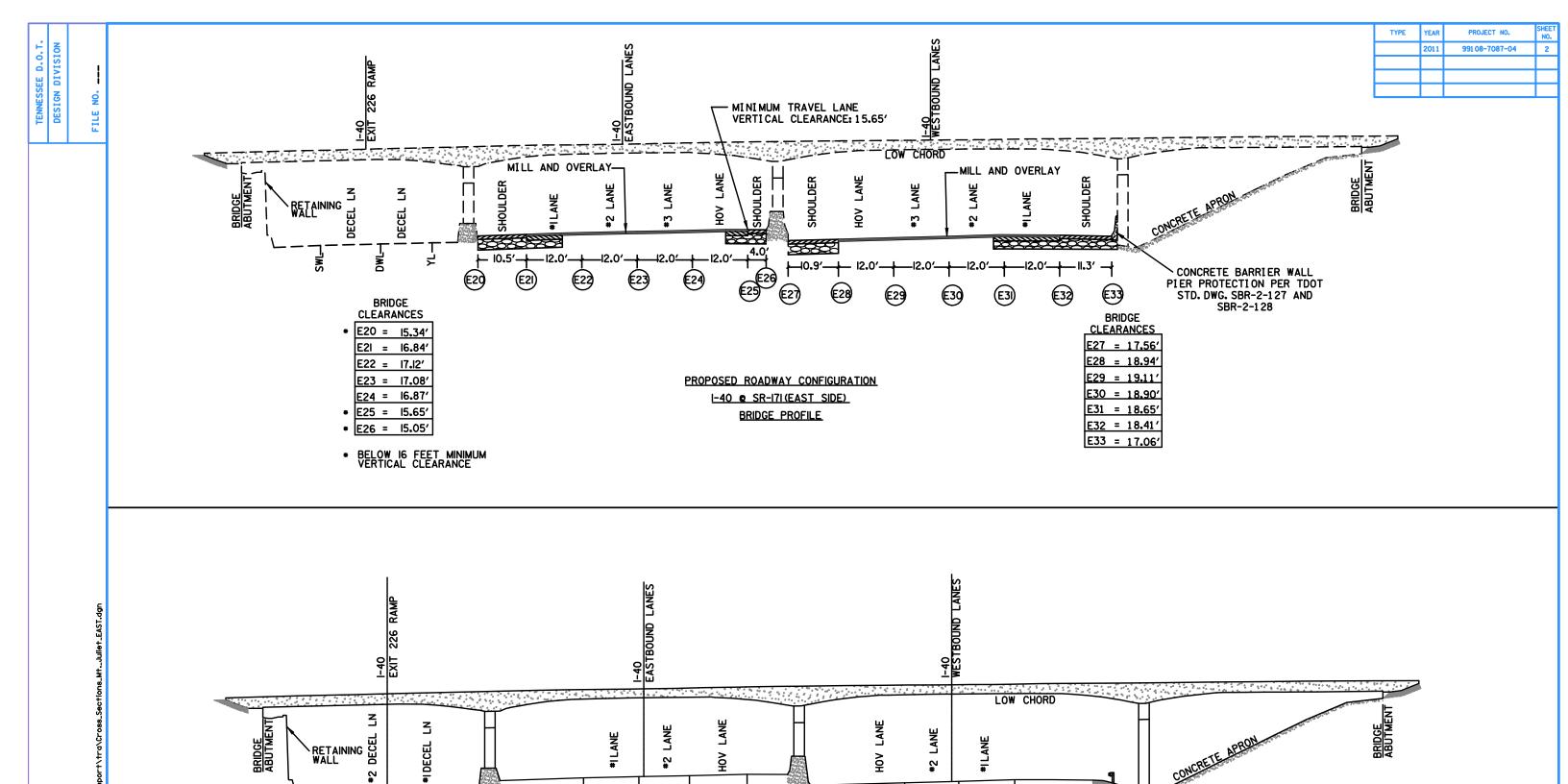
From Central Pike to East of SR-109 Wilson County, Tennessee

Appendix D

Bridge Profiles



SCALE: 1" = 20'



EXISTING ROADWAY CONFIGURATION I-40 @ SR-I7I (EAST SIDE) BRIDGE PROFILE

ᆛ

**E7** 

**E6** 

**E**5

SWL

**E4** 

BRIDGE CLEARANCES

EI = 18.34' E2 = 18.34' E3 = 17.46'

E4 = 17.07' E5 = 17.09'

E6 = 17.01'

E7 = 16.21'

E8 = 18.84'

E9 = 19.11'

EIO = 18.95'

EII = 18.68'

ă

**E2** 

(EI)

**E**3

**4**2

ě

(EIO)

(EII)

TMO E9

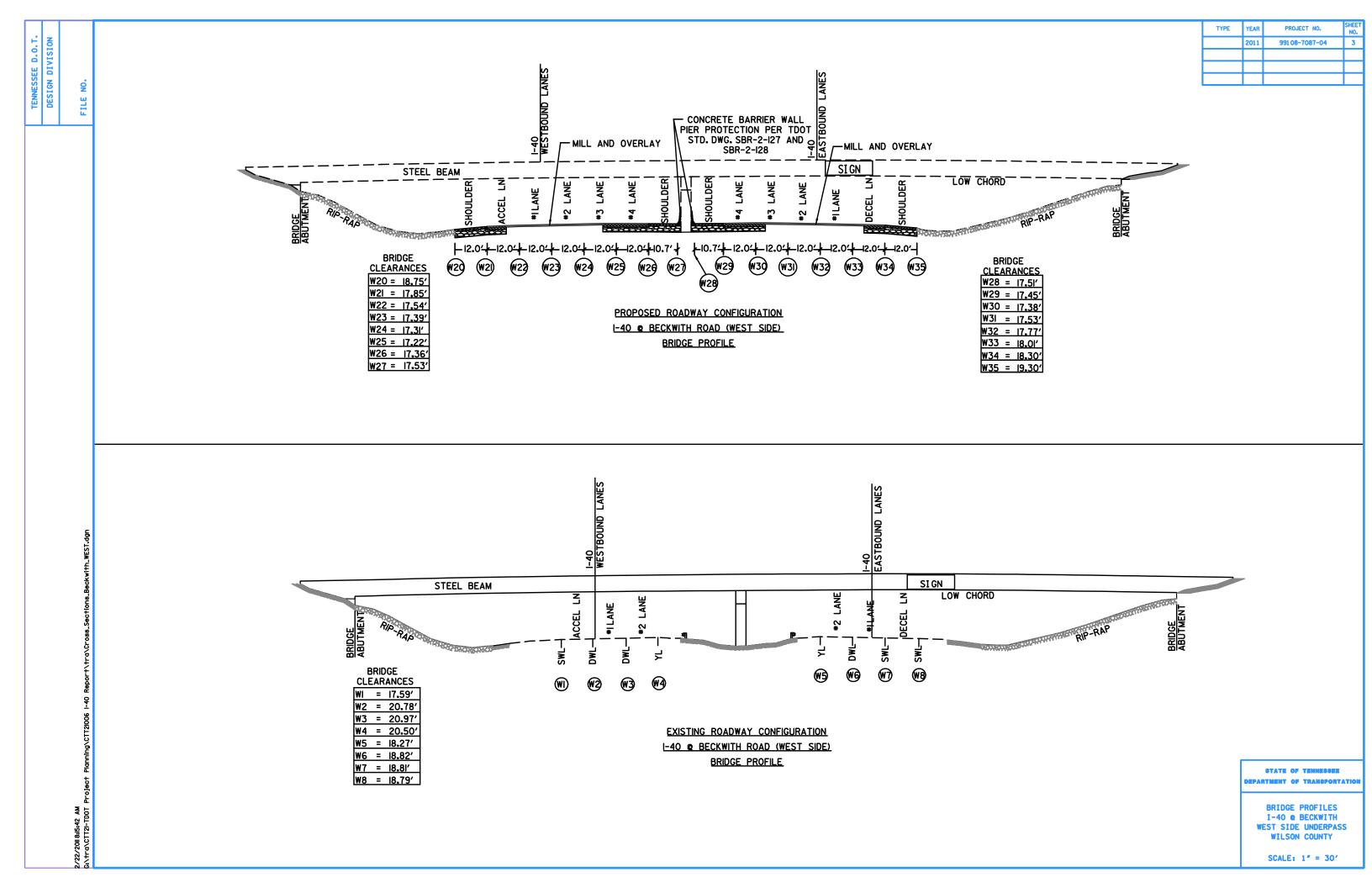
ᆛ

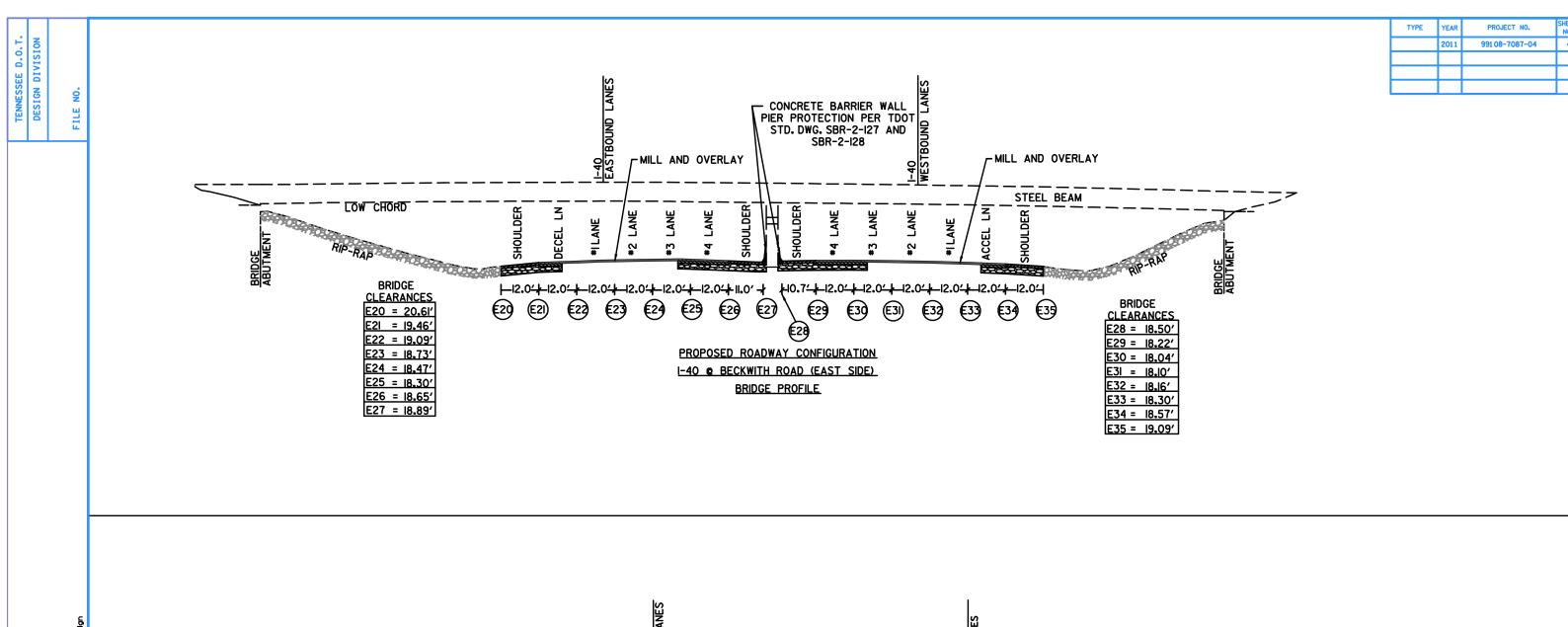
ÉB)

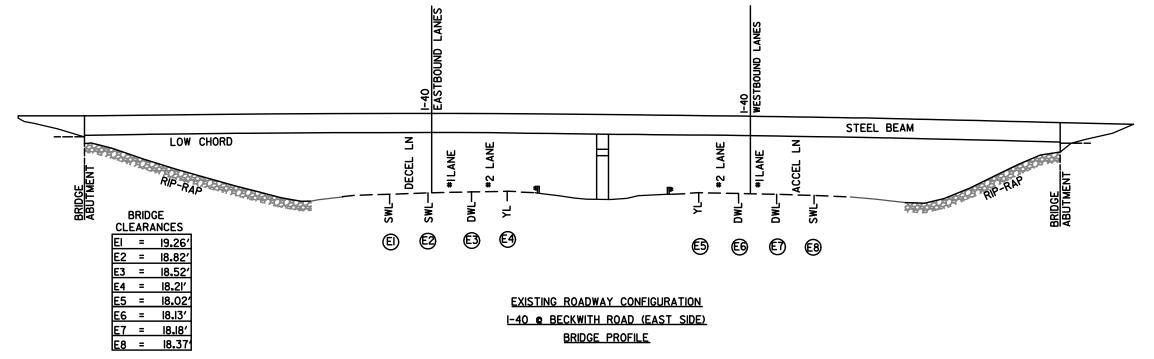
STATE OF TENNESSEE

**BRIDGE PROFILES** I-40 e SR-171 EAST SIDE UNDERPASS WILSON COUNTY

SCALE: 1" = 20'



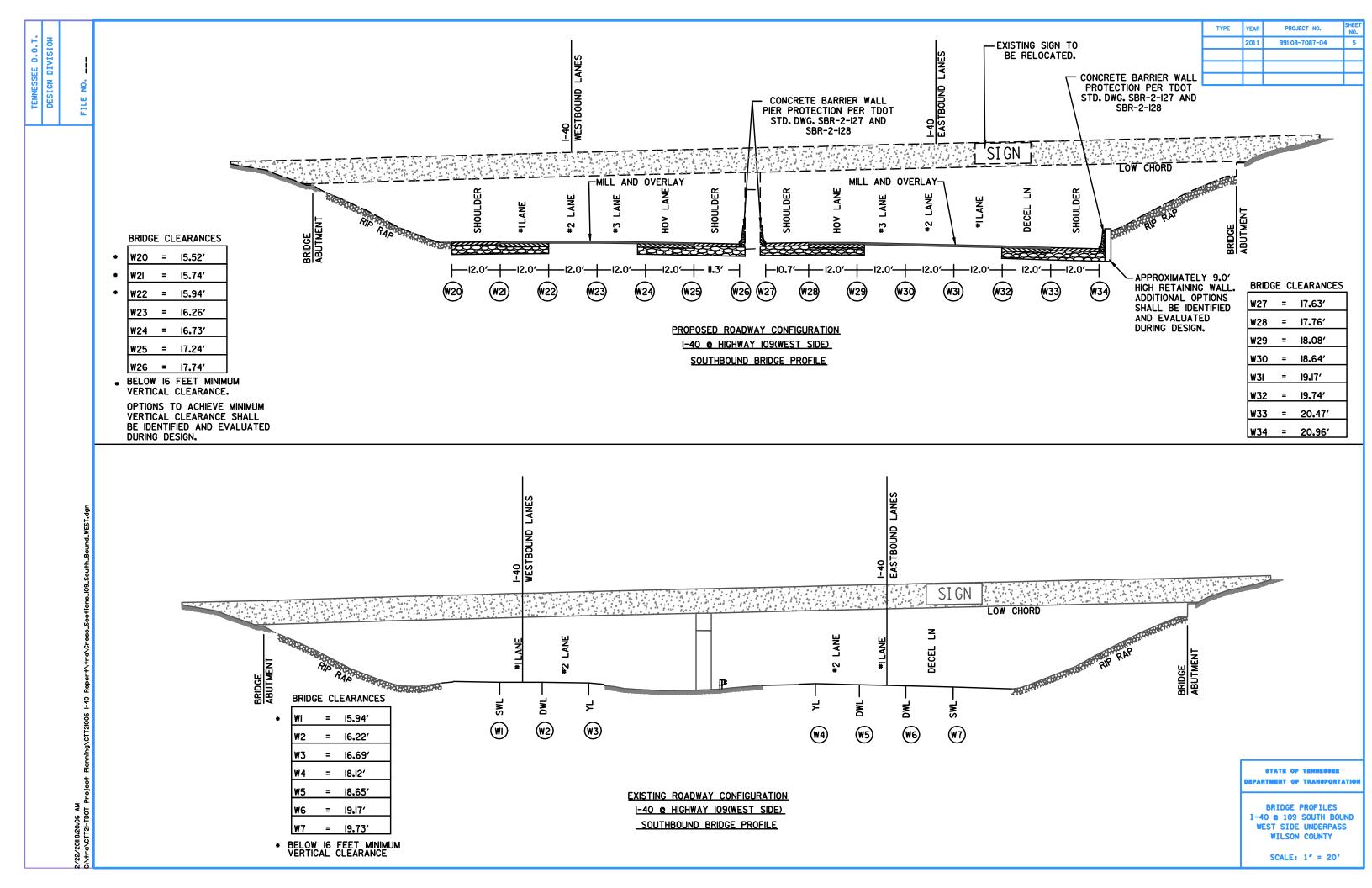


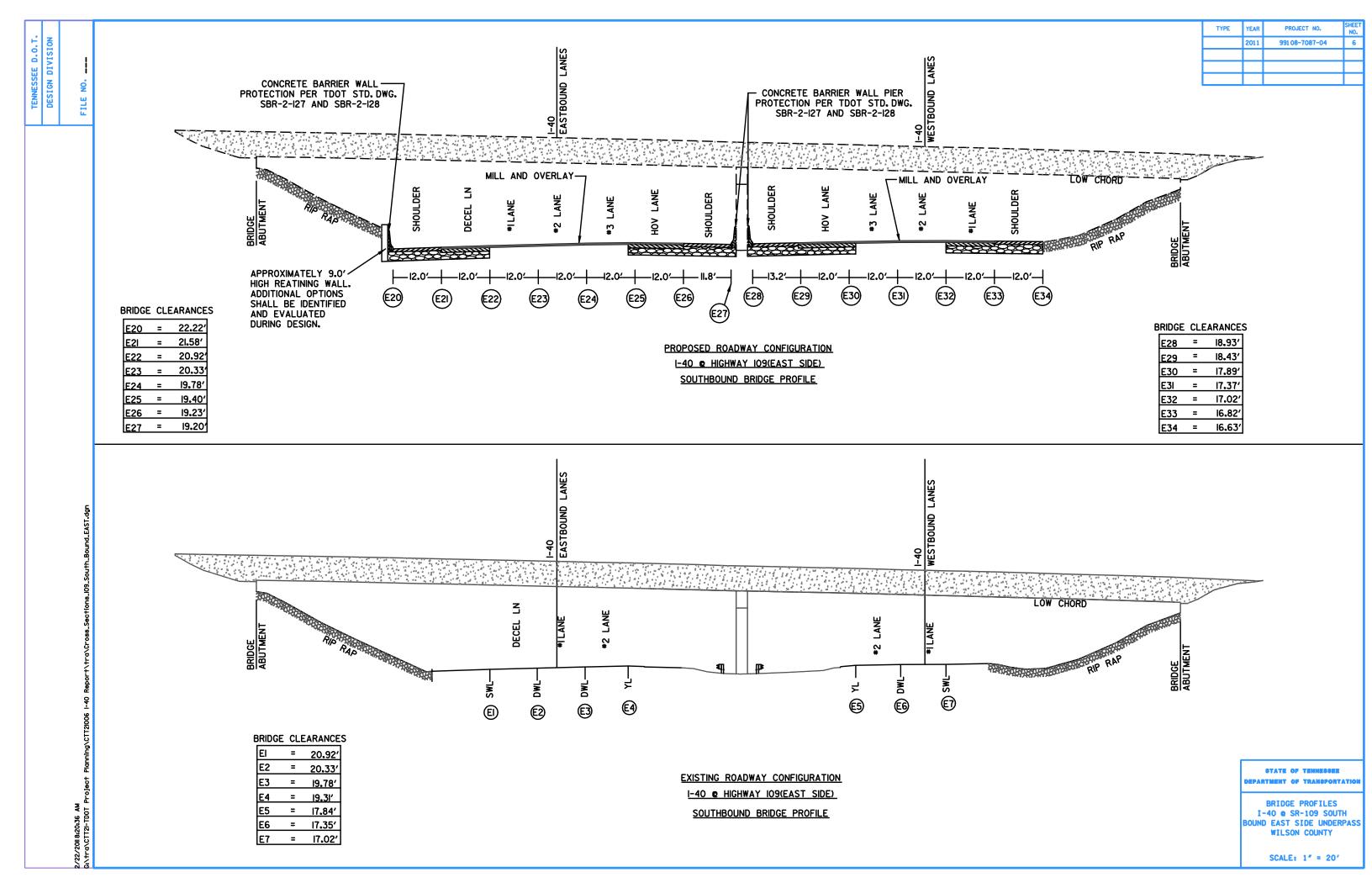


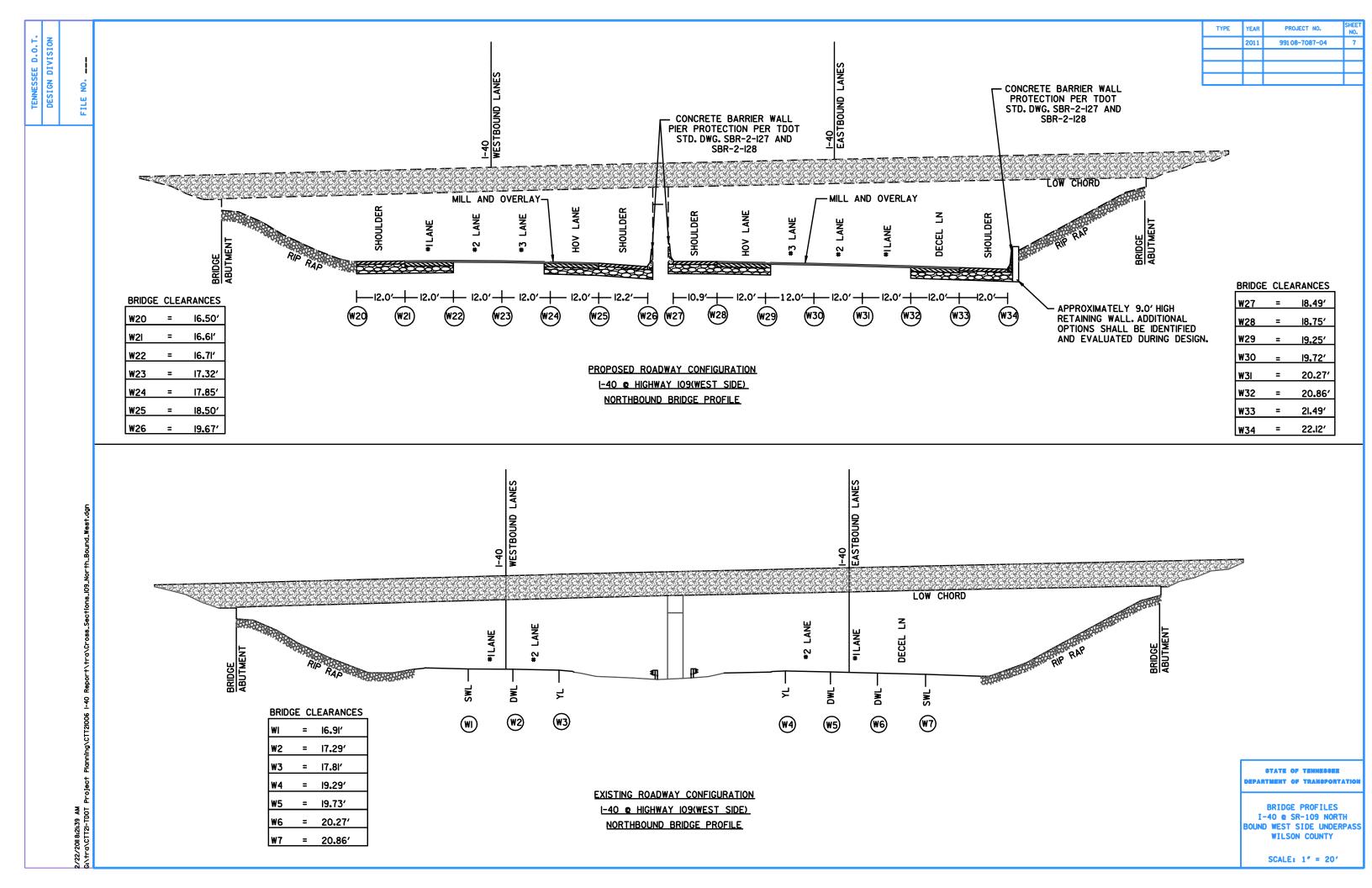
STATE OF TENNESSEE

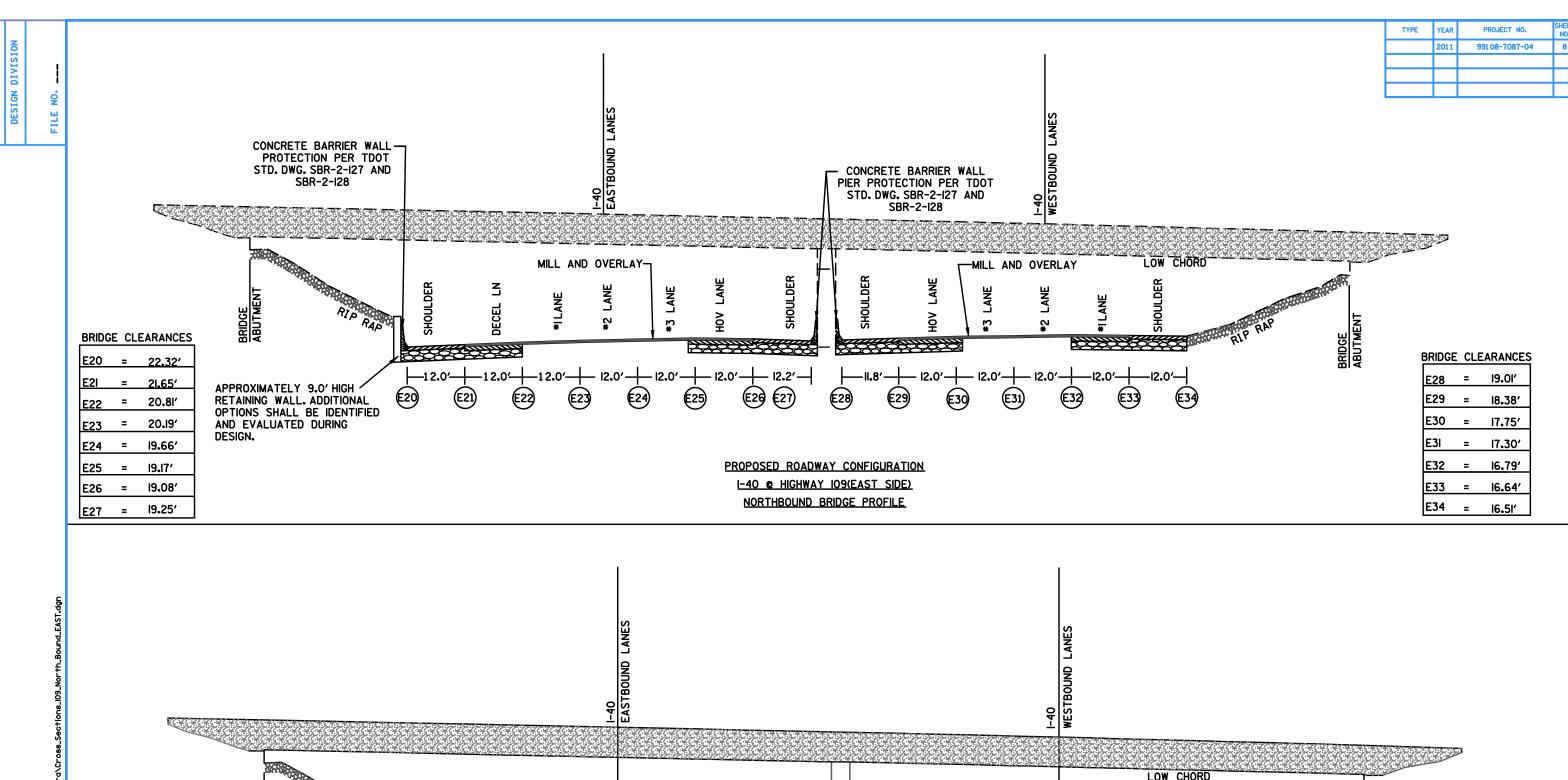
BRIDGE PROFILES
I-40 @ BECKWITH
EAST SIDE UNDERPASS
WILSON COUNTY

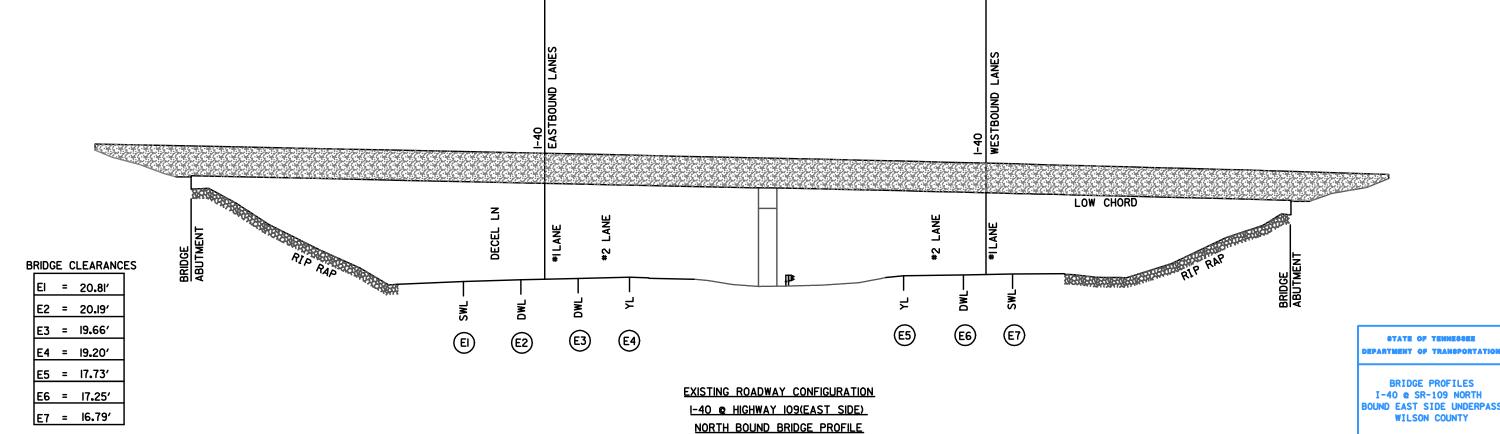
SCALE: 1" = 30'











SCALE: 1" = 20'

Technical Report

Project Description: Interstate 40 Lane Additions From Central Pike to East of SR-109

Wilson County, Tennessee

Appendix E

Capacity Analysis

# I40EB\_WestOfSR171\_Restricted\_AM.txt

HCS+: Basic Freeway Segments Release 5.2

_		
Oper	ational Analysis	
	IS 2011 Eastbound of SR 171	ne.
Flow	Inputs and Adjustments	
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type:     Grade     Segment length Trucks and buses PCE, ET Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population factor, fp Flow rate, vp	2576 0.90 716 12 0 Level 0.00 0.00 1.5 1.2 0.943 1.00	veh/h v % % mi pc/h/ln
•	d Inputs and Adjustment	• , ,
Lane width Right-shoulder lateral cleara Interchange density Number of lanes, N Free-flow speed:     FFS or BFFS Lane width adjustment, fLW Lateral clearance adjustment, Interchange density adjustmen Number of lanes adjustment, f Free-flow speed, FFS	12.0 6.0 0.50 3 Measured 70.0 0.0 6LC 0.0 t, fID 0.0 70.0 Urban Fr	ft ft interchange/mi  mi/h mi/h mi/h mi/h mi/h mi/h mi/h
LOS	and Performance Measure	S
Flow rate, vp Free-flow speed, FFS Average passenger-car speed, Number of lanes, N Density, D Level of service, LOS	1011 70.0 70.0 3 14.4 B	pc/h/ln mi/h mi/h pc/mi/ln

# I40EB\_WestOfSR171\_Restricted\_PM.txt

HCS+: Basic Freeway Segments Release 5.2

Operational Analysis			
Analyst: McKaig Agency or Company: ARCADIS Date Performed: 1/27/2011 Analysis Time Period: P.M. Freeway/Direction: I-40 Eastbound From/To: West of SR 171 Analysis Year: 2033 Description: Assumed: 15% of total volume in HOV lane.			
Flow Inputs and	Adjustments		
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type:     Grade     Segment length Trucks and buses PCE, ET Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population factor, fp Flow rate, vp	6035 0.90 1676 12 0 Level 0.00 0.00 1.5 1.2 0.943 1.00 2369	<pre>veh/h v % % mi  pc/h/ln</pre>	
Speed Inputs and	Adjustments		
Lane width Right-shoulder lateral clearance Interchange density Number of lanes, N Free-flow speed: FFS or BFFS Lane width adjustment, fLW Lateral clearance adjustment, fLC Interchange density adjustment, fID Number of lanes adjustment, fN Free-flow speed, FFS	12.0 6.0 0.50 3 Measured 70.0 0.0 0.0 0.0 3.0 70.0 Urban Freeway	ft ft interchange/mi  mi/h mi/h mi/h mi/h mi/h mi/h mi/h	
LOS and Performa	nce Measures		
Flow rate, vp Free-flow speed, FFS Average passenger-car speed, S Number of lanes, N Density, D Level of service, LOS	2369 70.0 54.5 3 43.4 E	pc/h/ln mi/h mi/h pc/mi/ln	

# I40WB\_WestOfSR171\_Restricted\_AM.txt

HCS+: Basic Freeway Segments Release 5.2

Operational Analysis			
Analyst: McKaig Agency or Company: ARCADIS Date Performed: 1/27/2011 Analysis Time Period: A.M. Freeway/Direction: I-40 Westbound From/To: West of SR 171 Analysis Year: 2033 Description: Assumed: 15% of total volume in HOV lane.			
Flow Inputs a	nd Adjustments		
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Segment length Trucks and buses PCE, ET Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population factor, fp Flow rate, vp  6221 veh/h 0.90  Recleation 1728 V  W  K  V  Trucks on buses 12  W  %  N  W  N  W  N  W  N  N  N  N  N  N  N			
Speed Inputs	and Adjustments		
Lane width Right-shoulder lateral clearance Interchange density Number of lanes, N Free-flow speed: FFS or BFFS Lane width adjustment, fLW Lateral clearance adjustment, fLC Interchange density adjustment, fID Number of lanes adjustment, fN Free-flow speed, FFS	12.0 6.0 0.50 3 Measured 70.0 0.0 0.0 0.0 0.0 3.0 70.0 Urban Freeway	ft ft interchange/mi  mi/h mi/h mi/h mi/h mi/h mi/h mi/h	
LOS and Perfo	ormance Measures		
Flow rate, vp Free-flow speed, FFS Average passenger-car speed, S Number of lanes, N Density, D Level of service, LOS	2442 70.0 3 F	<pre>pc/h/ln mi/h mi/h pc/mi/ln</pre>	

#### I40WB\_WestOfSR171\_Restricted\_PM.txt

HCS+: Basic Freeway Segments Release 5.2
\_\_\_\_\_Operational Analysis\_\_\_\_\_

Operational Analysis				
Analyst: McKaig Agency or Company: ARCADIS Date Performed: 1/27/2011 Analysis Time Period: P.M. Freeway/Direction: I-40 Westbound From/To: West of SR 171 Analysis Year: 2033 Description: Assumed: 15% of total volume in HOV lane.				
Flow I	nputs and Adjustments			
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type:     Grade     Segment length Trucks and buses PCE, ET Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population factor, fp Flow rate, vp	3409 0.90 947 12 0 Level 0.00 0.00 1.5 1.2 0.943 1.00 1338	<pre>veh/h v % % mi pc/h/ln</pre>		
Speed :	Inputs and Adjustments			
Lane width Right-shoulder lateral clearance Interchange density Number of lanes, N Free-flow speed: FFS or BFFS Lane width adjustment, fLW Lateral clearance adjustment, fI Interchange density adjustment, Number of lanes adjustment, fN Free-flow speed, FFS	0.50 3 Measured 70.0 0.0 LC 0.0	ft ft interchange/mi  mi/h mi/h mi/h mi/h mi/h mi/h mi/h		
LOS and Performance Measures				
Flow rate, vp Free-flow speed, FFS Average passenger-car speed, S Number of lanes, N Density, D Level of service, LOS	1338 70.0 70.0 3 19.1 C	<pre>pc/h/ln mi/h mi/h pc/mi/ln</pre>		

# I40EB\_SR171\_Diverge\_Restricted\_AM.txt

HCS+: Ramps and Ramp Junctions Release 5.2

Diverge	Analysis		
Analyst: McKaig Agency/Co.: ARCADIS Date performed: 1/26/2011 Analysis time period: A.M. Peak Freeway/Dir of Travel: I-40 Eastbound Junction: SR 171 Analysis Year: 2033 Description: Assumed: 15% of total volu	me in HOV lane.		
Freewa	y Data		
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Diverge 3 55.0 2576	mph vph	
Off Ram	p		
Side of freeway Number of lanes in ramp Free-Flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 2 35.0 323 780 0	mph vph ft ft	
Adjacent Ramp D	ata (17 one exists	)	· · · · · · · · · · · · · · · · · · ·
Does adjacent ramp exist? Volume on adjacent ramp Position of adjacent ramp Type of adjacent ramp Distance to adjacent ramp	No	vph ft	
Conversion to pc/h U	nder Base Conditio	ns	
	reeway Ramp		Adjacent
Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Length Trucks and buses PCE, ET Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population factor, fP Flow rate, vp	evel Level .00 % 0.00 .00 mi 0.00 .5 1.5 .2 1.2 .943 0.943 .00 1.00 034 380	% mi	Ramp  vph  v  %  %  mi
	12 Diverge Areas		
EQ	ion 25-8 or 25-9)		
P = 0.450 Using FD	Equation 0		

Page 1

# I40EB\_SR171\_Diverge\_Restricted\_AM.txt v = v + (v - v ) P = 1574 pc/h 12 R F R FD

\_\_\_Capacity Checks\_\_ Actual Maximum LOS F? 3034 6750 No Fi 1574 4400 No 2654 6750 No FO 380 3800 No R \_\_\_\_Level of Service Determination (if not F)\_\_\_\_\_ D = 4.252 + 0.0086 v - 0.009 L RDensity, pc/mi/ln Level of service for ramp-freeway junction areas of influence A \_\_\_\_\_Speed Estimation\_\_ Intermediate speed variable, D = 0.462Space mean speed in ramp influence area, S = 49.0mph Space mean speed in outer lanes, S = 58.5mph Space mean speed for all vehicles, S = 53.2mph

# I40EB\_SR171\_Diverge\_Restricted\_PM.txt

HCS+: Ramps and Ramp Junctions Release 5.2

Diverge An	alysis		
Analyst: McKaig Agency/Co.: ARCADIS Date performed: 1/26/2011 Analysis time period: P.M. Peak Freeway/Dir of Travel: I-40 Eastbound Junction: SR 171 Analysis Year: 2033 Description: Assumed: 15% of total volume	in HOV lane.		
Freeway D	ata		
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Diverge 3 55.0 6035	mph vph	
Off Ramp D	ata		
Side of freeway Number of lanes in ramp Free-Flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 2 35.0 1204 780 0	mph vph ft ft	
Adjacent Ramp Data	(if one exists	)	
Does adjacent ramp exist? Volume on adjacent ramp Position of adjacent ramp Type of adjacent ramp Distance to adjacent ramp	No	vph ft	
Conversion to pc/h Unde	r Base Conditio	ns	
Volume, V (vph) Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Length Trucks and buses PCE, ET Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population factor, fP Flow rate, vp	0.90 334 12 0 1 Level % 0.00 mi 0.00 1.5 1.2 3 0.943 1.00 1418	% mi	Adjacent Ramp vph v % % % mi
	25-8 or 25-9)		
EQ P = 0.450 Using Equ FD			

Page 1

# I40EB\_SR171\_Diverge\_Restricted\_PM.txt v = v + (v - v ) P = 3978 pc/h 12 R F R FD

\_\_\_Capacity Checks\_\_ Actual Maximum LOS F? 7108 6750 Yes Fi 3978 4400 No 6750 5690 No FO 1418 3800 No R \_\_\_Level of Service Determination (if not F)\_\_\_\_ D = 4.252 + 0.0086 v - 0.009 L = 12 Dpc/mi/ln Density, Level of service for ramp-freeway junction areas of influence F \_\_\_\_Speed Estimation\_\_ Intermediate speed variable, D = 0.556Space mean speed in ramp influence area, S = 47.8mph Space mean speed in outer lanes, S = 52.0mph Space mean speed for all vehicles, S = 49.6mph

# I40WB\_SR171\_Merge\_Restricted\_AM.txt

HCS+: Ramps and Ramp Junctions Release 5.2

Merge	Analysis		
Analyst: MCKaig Agency/Co.: ARCADIS Date performed: 1/28/2011 Analysis time period: A.M. Freeway/Dir of Travel: I-40 Westbound Junction: SR 171 Analysis Year: 2033 Description: Assumed: 15% of total vol	ume in HOV lane.		
Freew	ay Data		
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Merge 3 70.0 mph 3567 vph		
On Ra	mp Data		
Side of freeway Number of lanes in ramp Free-flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 2 35.0 mph 3122 vph 500 ft 1500 ft		
Adjacent Ramp	Data (if one exists)		
Does adjacent ramp exist? Volume on adjacent Ramp Position of adjacent Ramp Type of adjacent Ramp Distance to adjacent Ramp	No vph ft		
Conversion to pc/h	Under Base Conditions		
Volume, V (vph) Peak-hour factor, PHF	Freeway Ramp Adjacent Ramp 3567 3122 vph 0.90 0.90 991 867 v		
Trucks and buses Recreational vehicles Terrain type: Grade Length	12		
Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population factor, fP	1.5		
Estimation of V12 Merge Areas			
EQ	tion 25-2 or 25-3) Equation 0		
V = V (P) = 2332	·		

# I40WB\_SR171\_Merge\_Restricted\_AM.txt 12 F FM

	Capacity	Checks		
V FO	Actual 7878	Maximum 7200	LOS F? Yes	
R12	6009	4600	Yes	
Level of	Service Determ	ination (if not	F)	
Density, D = 5.475 + 0.007 R Level of service for ramp-	R	12	Α	pc/mi/ln
	Speed Estim	ation		
Intermediate speed variabl	e,	$M_{c} = 1.734$		
Space mean speed in ramp i	nfluence area,	S = 21.5	mph	
Space mean speed in outer	lanes,	$S_{0}^{R} = 65.1$	mph	
Space mean speed for all v	ehicles,	s = 25.5	mph	

# I40WB\_SR171\_Merge\_Restricted\_PM.txt

HCS+: Ramps and Ramp Junctions Release 5.2

Mer	ge Analysis	
Analyst: Agency/Co.: Date performed: Analysis time period: Freeway/Dir of Travel: Junction: Analysis Year: Description: Assumed: Analysis		
Fr	eeway Data	
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Merge 3 70.0 2212	mph vph
Or	n Ramp Data	
Side of freeway Number of lanes in ramp Free-flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 2 35.0 1408 500 1500	mph vph ft ft
Adjacent Ra	amp Data (if one exist	:s)
Does adjacent ramp exist? Volume on adjacent Ramp Position of adjacent Ramp Type of adjacent Ramp Distance to adjacent Ramp	No	vph
Conversion to po	:/h Under Base Conditi	ons
Junction Components  Volume, V (vph)  Peak-hour factor, PHF  Peak 15-min volume, v15  Trucks and buses	Freeway Ramp  2212 1408 0.90 0.90 614 391 12 12	Adjacent Ramp vph V
Recreational vehicles Terrain type: Grade Length Trucks and buses PCE, ET	0 0 Level Level % mi 1.5 1.5	% % mi mi
Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population factor, fP Flow rate, vp	1.2 0.943 1.00 2605 1.00 1.00 1.00	pcph
	of V12 Merge Areas	
L = (E EQ	Equation 25-2 or 25-3)	
P = 0.555 Us FM	sing Equation 0	
V = V (P) = 1	L446 pc/h Page 1	

# I40WB\_SR171\_Merge\_Restricted\_PM.txt 12 F FM

Capacity Checks				
V F0	Actual 4263	Maximum 7200	LOS F? No	
v R12	3104	4600	No	
Level of	Service Determ	ination (if no	ot F)	
Density, D = $5.475 + 0.00734 + 0.0078 + 0.0078 + 0.00627 + 0.006$				pc/mi/ln
	Speed Estim	ation		
Intermediate speed variabl	e,	$M_{c} = 0.23$	33	
Space mean speed in ramp i	nfluence area,	S S = 63.5 R	mph	
Space mean speed in outer	lanes,	S = 67.6	5 mph	
Space mean speed for all v	ehicles,	S = 64.6	5 mph	

# I40EB\_Weaving\_Restricted\_AM.txt

# HCS+: Freeway Weaving Release 5.2

Operatio	onal Analysis	S		
Analyst: Agency/Co.: Date Performed: Analysis Time Period: Analysis Time Period: Freeway/Dir of Travel: Weaving Location: Analysis Year: Description: Assumed:  MCKaig ARCADIS 1/27/2011 A.M. Freeway/Dir of Travel: SR 109 to Truck 2033 Description: Assumed: 15% of total vo	k Park	lane.		
Inp	outs			
Freeway free-flow speed, SFF Weaving number of lanes, N Weaving segment length, L Terrain type Grade Length Weaving type Volume ratio, VR Weaving ratio, R	70 3 1340 Level A 0.32 0.03	mph ft % mi		
Conversion to pc/h	Under Base (	Conditions		
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Trucks and buses PCE, ET Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population adjustment, fP Flow rate, v	1748 0 0.90 0.9 486 0 12 4 0 0 1.5 1.5 1.2 1.2 0.943 0.9 1.00 1.0 2058 0	V -D A-D 23 90 0.90 6 25 0 5 1.5 2 1.2 980 0.889 00 1.00 28	V B-C 791 veh 0.90 220 v 12 % 0 % 1.5 1.2 0.943 1.00 931 pc/	h/h /h
Weaving and Non	ı-Weaving Spe	eeds		
a (Exhibit 24-6) b (Exhibit 24-6) c (Exhibit 24-6) d (Exhibit 24-6) Weaving intensity factor, Wi Weaving and non-weaving speeds, Si Number of lanes required for unconstrained operation, Nw (Exhibit 2	Weaving 0.15 2.20 0.97 0.80 0.71 50.11	Non-Weaving 0.00 4.00 1.30 0.75 0.38 58.43	9	
Maximum number of lanes, Nw (max) (Exh Type of operation is	iibit 24-7)	1.40 Unconstrai	ned	
Weaving Segment Speed, Densit	y, Level of	Service and	Capacity	
Weaving segment speed, S Weaving segment density, D Level of service, LOS Capacity of base condition, cb	55.50 mph 18.12 pc/n B 5722 pc/h Page 1	mi/ln		

#### I40EB\_Weaving\_Restricted\_AM.txt

Capacity as a 15-minute flow rate, c 5398 pc/h Capacity as a full-hour volume, ch 4858 pc/h

\_\_\_\_Limitations on Weaving Segments\_\_

		If Max Exceeded See	
	Analyzed	Maximum	Note
Weaving flow rate, Vw	959	2800	a
Average flow rate (pcphpl)	1005	2400	b
Volume ratio, VR	0.32	0.45	С
Weaving ratio, R	0.03	N/A	d
weaving length (ft)	1340	2500	e
Notes:			

- Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- Capacity constrained by basic freeway capacity.
- Capacity occurs under constrained operating conditions. С.
- Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such
- Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h
- (Type A), 4,000 (Type B), 3,500 (Type C). Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such
- Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

# I40EB\_Weaving\_Restricted\_PM.txt

# HCS+: Freeway Weaving Release 5.2

Operational Analysis				
Analyst: McKaig Agency/Co.: ARCADIS Date Performed: 1/27/2011 Analysis Time Period: P.M. Freeway/Dir of Travel: I-40 Eastbound Weaving Location: SR 109 to Truck Park Analysis Year: 2033 Description: Assumed: 15% of total volume in HOV lane.				
Inp	uts			
Freeway free-flow speed, SFF Weaving number of lanes, N Weaving segment length, L Terrain type Grade Length Weaving type Volume ratio, VR Weaving ratio, R	70 3 1340 Level A 0.21 0.02	mph ft % mi		
Conversion to pc/h	Under Base (	Conditions		
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Trucks and buses PCE, ET Recreational vehicle PCE, ER Heavy vehicle adjustment, fHV Driver population adjustment, fP Flow rate, v	3451 0 0.90 0.9 959 0 12 4 0 0 1.5 1.5 1.2 1.2 0.943 0.9	V -D A-D 21 90 0.90 6 25 0 5 1.5 2 1.2	V B-C 872 veh/h 0.90 242 v 12 % 0 % 1.5 1.2	
Weaving and Non	-Weaving Spe	eeds		
a (Exhibit 24-6) b (Exhibit 24-6) c (Exhibit 24-6) d (Exhibit 24-6) Weaving intensity factor, Wi Weaving and non-weaving speeds, Si Number of lanes required for unconstrained operation, Nw (Exhibit 2 Maximum number of lanes, Nw (max) (Exh	Weaving 0.15 2.20 0.97 0.80 0.97 45.41 4-7) ibit 24-7)	Non-Weaving 0.00 4.00 1.30 0.75 0.53 54.18 0.91 1.40		
Type of operation is Unconstrained				
Weaving segment speed, S Weaving segment density, D Level of service, LOS Capacity of base condition, cb	52.11 mph	ni/ln	<u>-</u>	

#### I40EB\_Weaving\_Restricted\_PM.txt

Capacity as a 15-minute flow rate, c 5892 pc/h Capacity as a full-hour volume, ch 5303 pc/h

\_\_\_\_\_Limitations on Weaving Segments\_\_\_\_\_\_

		If Max Exce	eded See Note
	Analyzed	Maximum	Note
Weaving flow rate, Vw	1053	2800	a
Average flow rate (pcphpl)	1705	2400	b
Volume ratio, VR	0.21	0.45	С
Weaving ratió, R	0.02	N/A	d
Weaving length (ft)	1340	2500	ė
Notes:			

- a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
- b. Capacity constrained by basic freeway capacity.
- Capacity occurs under constrained operating conditions.
- d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
- e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
- f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
   g. Five-lane Type A segments do not operate well at volume ratios greater.
- g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
- h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
- Type C weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

Technical Report

Project Description: Interstate 40 Lane Additions From Central Pike to East of SR-109

Wilson County, Tennessee

# Appendix F

Design Exceptions



### STATE OF TENNESSEE DEPARTMENT OF TRANSPORTATION NASHVILLE, TENNESSEE 37243-1402

# **DESIGN EXCEPTION REQUEST AND JUSTIFICATION FORM**

TO:	_ , Division Administrator, FHWA
FROM:	_ ,
<b>DATE:</b> <u>1/31/2011</u>	
SUBJECT: Design Exception Request Project No. 99108-7087-0 PIN 114169.00 Project Description: I-40 L	
CONTROLLING CRITERIA FOR WHICH E	EXCEPTION IS REQUESTED:
Design Speed	ion 🔲 Bridge Width 🔲

#### **DESIGN EXCEPTION REQUESTED:**

Eastbound inside shoulder width along the Interstate 40 underpass at SR-171.

# **DESIGN DATA:**

	Standard for the Existing Poste	ctional Classification: <u>Int</u> he Above Classification ed Speed: <u>70 mph</u> in: <u>Flat to Rolling</u>			osted Speed: <u>70</u> an Area: <u>Urban</u>	mph_
	Traffic Data:	ADT (20 <u>11</u> ): <u>70,950</u> ADT (20 <u>31</u> ): <u>93,650</u> DHV: <u>8,051</u>		D: <u>60% -40</u> T: <u>18%</u> V:	<u>0%</u>	
DE	SIGN FEATUR	ES:				
			Standard	Existing	Proposed	N/A
	Cross Slop Supereleva Minimum R Minimum S	tion: adius of Curve:				
	Sight Dista					
	Vertical Cu					
	Vertical Cu Maximum (	rve:				$\boxtimes$
RO	ADWAY TYPIC	CAL SECTION:	Standard	Existing	Proposed	N/A
	Horizontal ( (other than Shoulder W	the clear zone)	10'			
		side Shoulders: de Shoulders: ı:	Paved 10' 12'	18.1' 8.6' 12'	10.3' 4.0' 12'	$\boxtimes$
BR	IDGE FEATUR	ES:	Standard	Existing	Proposed	N/A
		oulder Widths: ulder Widths:				
	Sufficiency Vertical Cle To \ To (	Rating:				

# **FACTORS CONSIDERED:**

1)	Accident experience or data  Data Available ☐ No Data Available ☐ Not Applicable ☐  ———
2)	Effect of the variance from the design standards on safety and operation of the facility  Effect considered No effect on the facility Not Applicable The only I-40 shoulder at the SR-171 underpass not meeting the minimum inside shoulder width of 10 feet and/or the minimum paved outside shoulder width of 10 feet is the east bound 4.0 feet wide inside shoulder. The impact to safety and operations should be minimum considering the narrow shoulder is located adjacent to an HOV lane and occurs only under the SR-171 overpass. The HOV lane is significant since an HOV lane typically carries less traffic during the peak hour than the regular travel lanes.
3)	Safety mitigation measures considered and provided  Measures provided ☑ Measures not justified □ Not Applicable □  There is an existing bridge barrier wall along the edge of the eastbound inside shoulder.
4)	Compatibility of the design and operation with adjacent sections  Considered ☐ Not a Consideration ☒ Not Applicable ☐  ———
5)	Comparative cost of the full standard versus the lower design proposed  Considered ☐ Not a Consideration ☐ Not Applicable ☒  Achieving the minimum 10 feet inside shoulder would require relocating an existing bridge pier. This would result in replacing the bridge which is cost prohibitive.
6)	Long term effect of the reduced design as compared to the full standard  Considered Not a Consideration Not Applicable  The long term effect should be minimal since an HOV lane carries a lower percentage of traffic than the three adjacent lanes.
7)	Difficulty obtaining the full standard such as right-of-way restriction, environmental impacts, etc.  Considered ☑ Not a Consideration ☐ Not Applicable ☐ The issue is the limited distance between the existing bridge piers. The only option is to relocate a bridge pier which would require complete bridge replacement.
8)	Capacity reductions or operational reductions caused by the design  Considered ☐ Not a Consideration ☐ Not Applicable ☐  Should have a limited impact on capacity since the shoulder is nearest the HOV lane and an HOV lane typically handles a lower percentage of the peak hour traffic.
9)	Level of service for the full standard versus the proposed design Considered ☐ Not a Consideration ☒ Not Applicable ☒
10)	Cumulative effect of more than one standard that is being reduced  Considered ☐ Not a Consideration ☐ Not Applicable ☐  ———
11)	Possibility of improving or achieving the full standard feature in the future  Applicable Not Applicable Not on the state highway system

The issue is the limited distance between the existing bridge piers. The only option is to relocate a bridge pier which would require complete bridge replacement.

#### **DESIGN EXCEPTION AND JUSTIFICATION:**

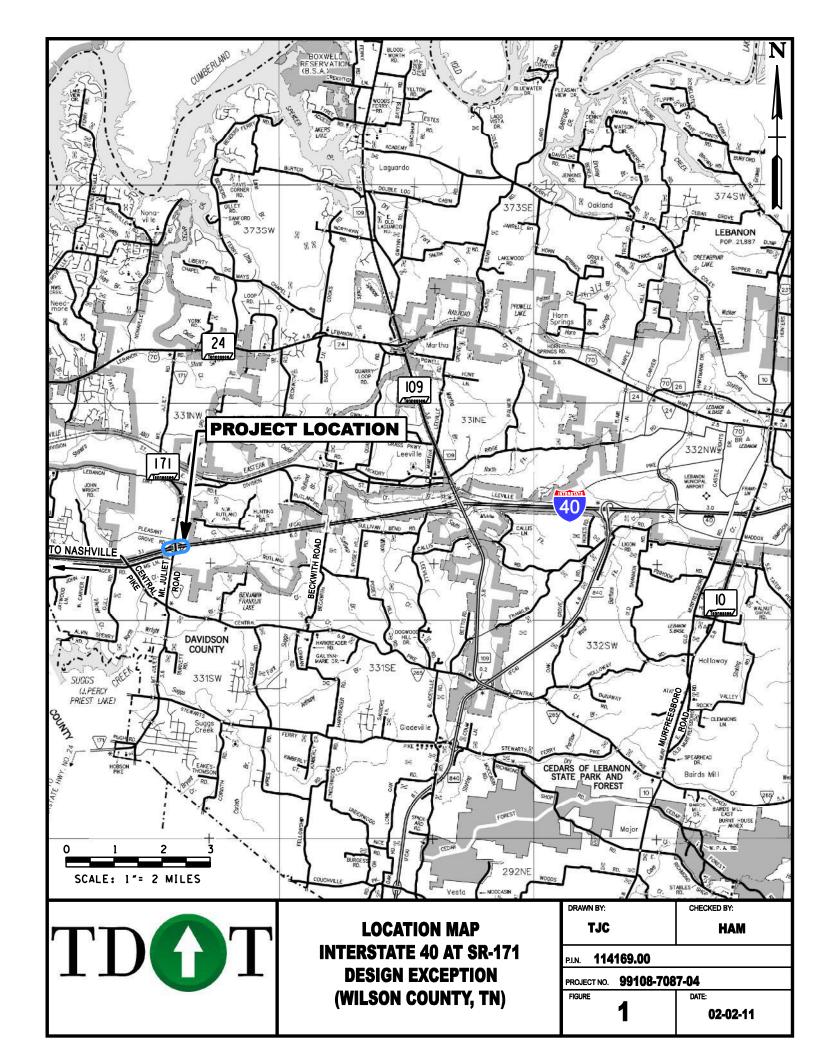
As stated above the eastbound I-40 inside shoulder at the SR-171 underpass does not meet the minimum shoulder width of 10 feet. A design exeception is requested for the 4.0 feet shoulder width. The limited shoulder width is caused by the location of the existing bridge piers. Relocating a bridge pier which would require a new bridge which is cost prohibitive. Additionally, the 4.0 feet shoulder is located adjacent to an HOV lane. HOV lanes typically carry less traffic which should minimize the impacts. See the attached conceptual plan and bridge profiles for additional information.

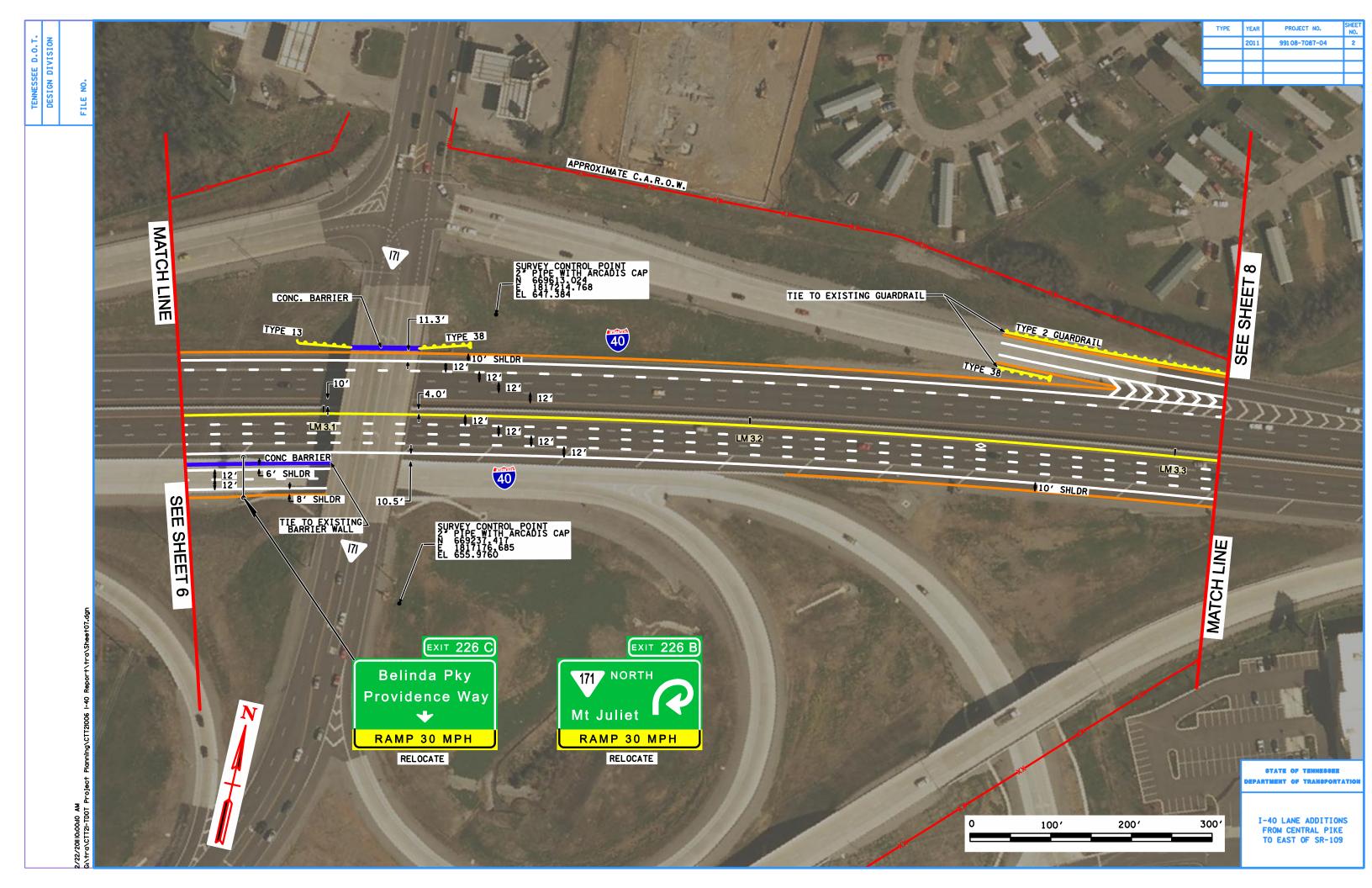
#### ATTACHMENTS:

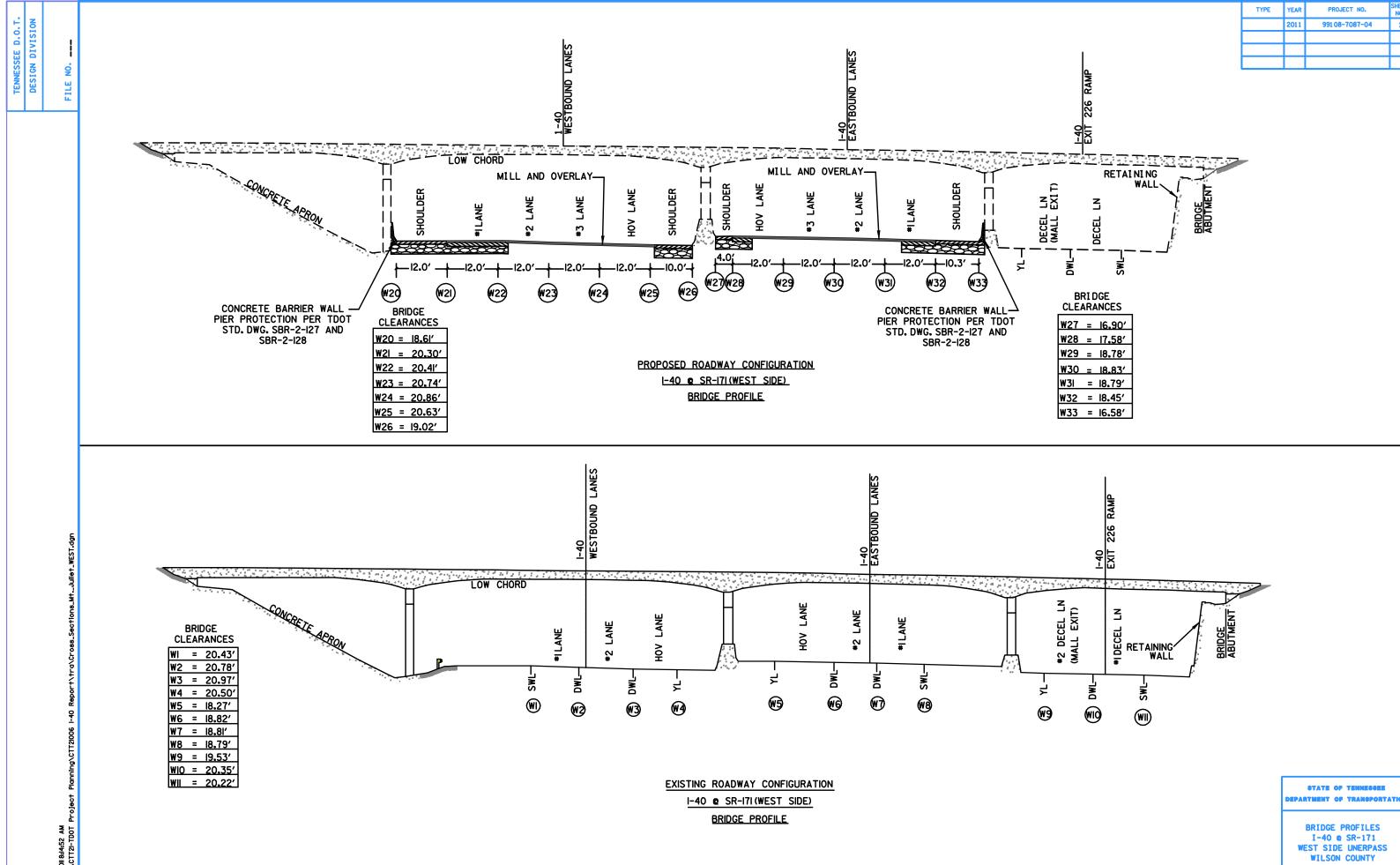
A location map, conceptual plan and bridge profiles showing both bridge faces along with existing and proposed roadway configurations are attached.

DESIGN EXCEPTION REVIEW COMMITTEE RECOMMENDATION FOR APPROVAL:				
	, Assistant Director, Design D	Division, Region 1		
	, Assistant Director, Design D	Division, Region 2		
	, Assistant Director, Design D	Division, Region 3		
	, Assistant Director, Design D	Division, Region 4		
Comments Attached				
APPROVED:				
Division Administrator	r, FHWA	Date		

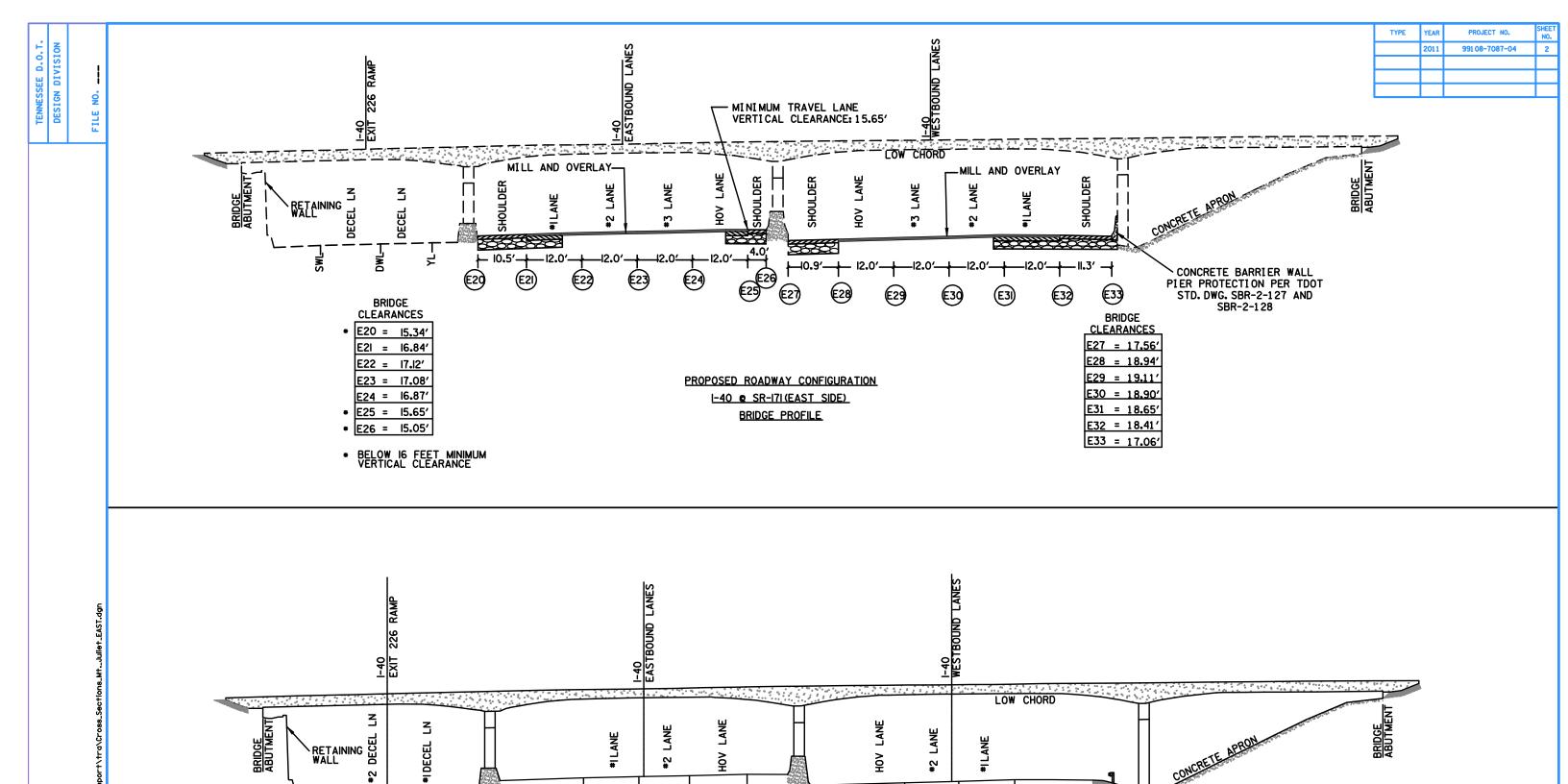
cc: Quality Assurance Section







SCALE: 1" = 20'



EXISTING ROADWAY CONFIGURATION I-40 @ SR-I7I (EAST SIDE) BRIDGE PROFILE

ᆛ

**E7** 

**E6** 

**E**5

SWL

**E4** 

BRIDGE CLEARANCES

EI = 18.34' E2 = 18.34' E3 = 17.46'

E4 = 17.07' E5 = 17.09'

E6 = 17.01'

E7 = 16.21'

E8 = 18.84'

E9 = 19.11'

EIO = 18.95'

EII = 18.68'

ă

**E2** 

(EI)

**E**3

**4**2

ě

(EIO)

(EII)

TMO E9

Y

ÉB)

STATE OF TENNESSEE

**BRIDGE PROFILES** I-40 e SR-171 EAST SIDE UNDERPASS WILSON COUNTY

SCALE: 1" = 20'